

Engineering Report

November 3, 2022

Revised December 12, 2022

Revised January 6, 2023

Revised February 6, 2023

Revised February 10, 2023

Prepared For

Piage Management Corp

49 Plains Road

Essex, Connecticut 06426

Prepared By

Doane Engineering

P. O. Box 113

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1.0 Introduction:

This Engineering Report has been prepared on behalf of Piage Management Corp. who is seeking approval for the development of an approximately 2.0-acre parcel located in the central portion of Essex. The parcel is located at 49 Plains Road (Conn. Route 153) in the Town of Essex, Connecticut. Please see Figure 1 for a location map.

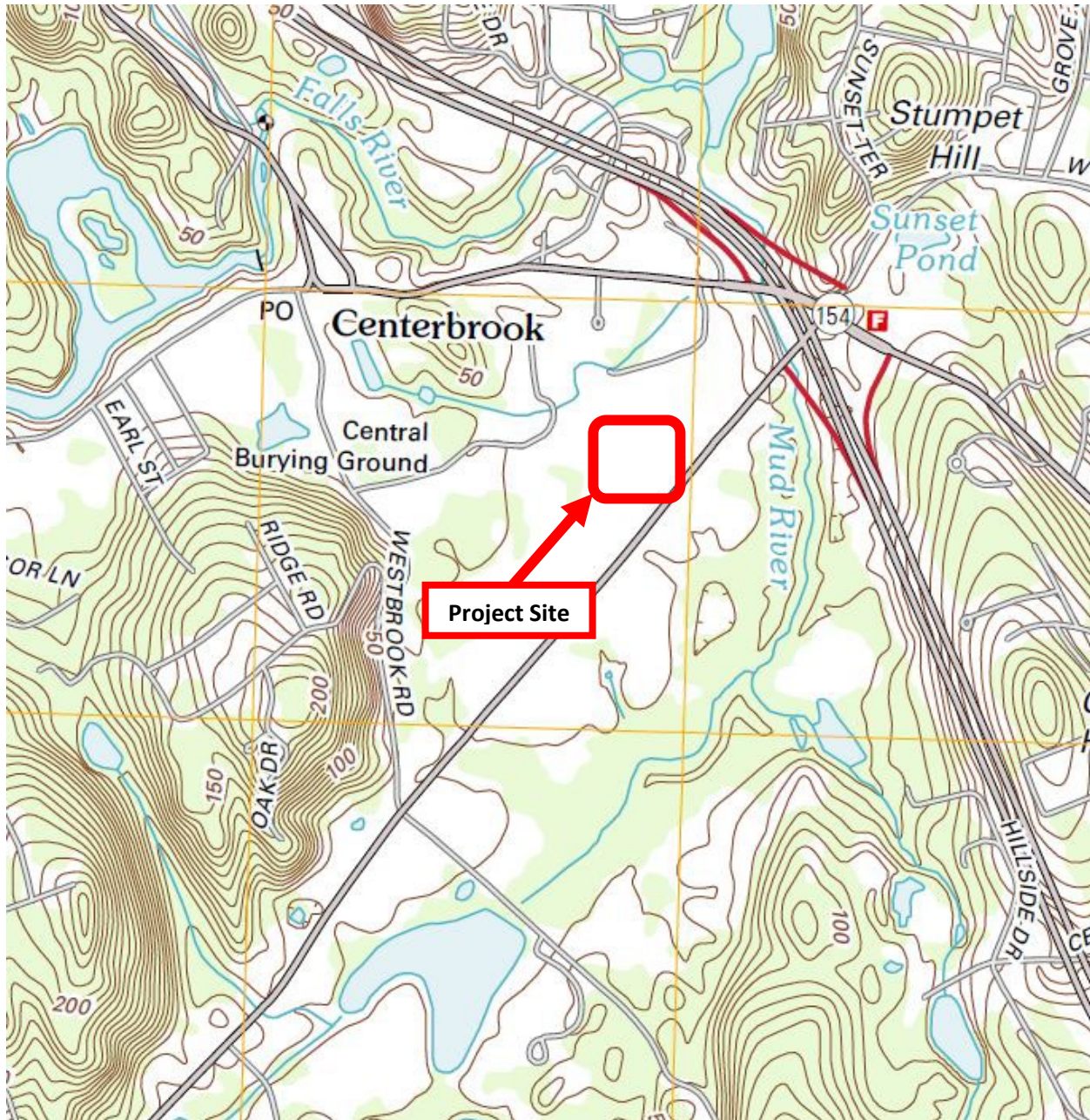


Figure 1. Project Location
U.S.G.S Essex Connecticut Quadrangle

The planned development proposal consists of the construction of a new 10,125 sf. warehouse to the rear of the existing 4,750 sf warehouse. Additional site improvements include driveways, parking areas, utility services, on-site wastewater system, landscaping, and stormwater management facilities.

The project site is located primarily within the Limited Industrial (LI) Zoning District. The access way to the site is in the Business District (B). The existing land used adjacent to and in the vicinity of the site is residential and commercial. The property is currently used as a warehouse that is occupied by Cross Island Provisions.

The site is served by the Connecticut Water Company public water system, public communication, and electric utilities located within the Plains Road Right of Way.

Surface runoff from the site currently drains to a small wetland located in the southeast corner of the site. This wetland discharges to a swale towards Plains Road and an 18" pipe crossing under Plains Road. The runoff ultimately discharges to the Mud River.

Surface runoff from the developed site will be collected by an on-site drainage system and detained in above ground and underground storage onsite. The proposed stormwater management system will continue to direct stormwater to the on-site wetland.

The site is not located within a flood zone per FIRM Community Panel Number 090065 0331 G map effective date 08-28-2008.

The site is located with the Water Resource Protection Area.

The site is not located within Aquifer Protection Area or identified Connecticut Department of Energy and Environmental Protection Diversity Database Area.

The Natural Resources Conservation Service Soil Survey of the State of Connecticut indicates that the uplands surficial soil type on the site is classified as Ninigret-Urban Land Complex 0%-5% Slopes (221A)

The site contains 0.06 acres of inland wetlands and 0.5 acres of upland review area.

The total area of land disturbance associated with the completed project construction activities is approximately 1.7 acres. The approximate area of disturbance within the upland review area is 0.4 acres.

2.0 Hydrologic Model Development:

The site stormwater management system has been designed in accordance with standard hydrologic and hydraulic engineering practices. HydroCAD Version 10.10 (Hydrologic Modeling Software 9 HydroCAD Software Solutions, LLC) was used to create the Hydrologic models and estimates of peak rates of discharge and volumes of runoff. The U.S. Department of Agriculture Soil Conservation Service (now Natural Resources Conservations Service) Technical Release 20 Computer Program for Project Formulation Hydrology Methodology was used within the HydroCAD software program. TR-20 is a single event, lumped parameter surface water hydrologic model that simulates the precipitation-runoff relationships of a drainage area. The model used the Soil Conservation Service Curve Number and Unit-Hydrograph Methods to represent infiltration losses and to transform excess precipitation into runoff, and the Modified Plus (Storage-Indication) Method to perform reservoir routing.

NOAA Precipitation Frequency Atlas 14 for the Northeastern States 24-hour rainfall depths in the project site vicinity shown in Table 1 were accessed from the NOAA Precipitation Frequency Data Server and entered into the model.

Table 1
24-Hour Rainfall Depths for the Project Site Vicinity

Recurrence Interval Year	Rainfall Depth Inches
2	3.44
10	5.20
25	6.31
50	7.13
100	8.01

Partial duration series precipitation frequency data was also accessed from the NOAA Precipitation Frequency Data Server and entered into the models to create a synthetic rainfall distribution specific to the project site vicinity.

Catchment area boundaries were delineated using the existing conditions mapping for the site. The delineations were checked and adjusted based on a field inspection.

Antecedent Moisture Condition II was used to represent the soil moisture condition in the catchment areas prior to the modeled rainfall events.

3.0 Stormwater Management System:

The site stormwater management system consists of an underground stormwater detention area, 2 above ground detention areas, and the associated collection system. The system has been designed to reduce the peak discharge from the site.

The storm drainage pipes have been sized to accommodate the 25-year storm. All discharges from the stormwater detention basins have been sized to accommodate the 100 year storm.

The above ground stormwater basins and underground infiltration basin have been designed to meet the water quality volume and annual groundwater recharge volume requirements of the Connecticut Department of Energy and Environmental Protection Stormwater Quality Manual for the developed site.

All catch basins will have 4' sumps and hooded outlets to help with debris collection and water quality.

Infiltration will be used in the underground basin. The basin has been designed to have 3' separation from the seasonal high ground water level (infiltration tests have been performed with the results shown in appendix H). A layer of unstable material was found from approximately elevation 36.0' to 34.0'. This material will be removed from the area under the infiltration basin and replaced with a material having a minimum infiltration rate of 4" per hour. To include a factor of safety, an infiltration rate of 1" per hour has been assumed in the storm water model.

The storm water management system provides attenuation of the peak discharge rates in addition to matching the 24 hour total storm water discharge from the developed site. A summary of the rates of peak discharge, reservoir elevations, and the 24-hour storm water flows are shown below.

Table 2: Peak Discharge Reservoir Elevations and 24-Hour Storm Water Flows

Storm	Existing Peak(cfs)	Proposed Peak (cfs)	Change Peak (cfs)	Existing 24 Hour(af)	Proposed 24 Hour (af)	Change 24 Hour (af)	Basin 20S Elevation	Basin 21SA Elevation	Underground 22SB Elevation
1 Year	1.76	0.46	-1.3	0.14	0.13	-0.01	33.64	37.42	37.61
2 Year	2.35	0.96	-1.39	0.20	0.19	-0.01	33.8	37.43	37.74
5 Year	3.37	1.95	-1.42	0.30	0.29	0.00	34.07	37.43	38
10 Year	4.25	2.59	-1.66	0.39	0.38	-0.01	34.26	37.44	38.28
25 Year	5.49	3.15	-2.34	0.53	0.52	0.00	34.49	37.45	38.5
50 Year	6.42	4.57	-1.85	0.63	0.63	0.00	34.63	37.45	38.58
100 Year	7.41	6.64	-0.77	0.75	0.75	0.00	34.74	37.46	38.67

5.0 Sanitary System Design Information:

The sanitary system has been designed based on water usage data collected from June 2019 through September 2022 by Connecticut Water Company. The calculated daily use of the existing warehouse business is 144 gallons per day. This is calculated excluding 2 outlying data points on 3/7/2022 and 3/9/2022. It is assumed that a leak caused this data to not be in line with the other water data collected.

Cross Island Provisions currently has 15 employees, 8 office staff and 7 delivery drivers that are on the road the majority of the day. Upon completion of the new warehouse building, it is estimated that the business will have 30 employees. Based on the average daily water usage of 144 gallons and the current number of employees (15) it is estimated that 10 gpd are generated by each employee.

$$144 \text{ gallons} / 15 \text{ employees} = 10 \text{ gallons per day per employee}$$

A safety factor of 1.5 can then be applied, bringing the 10 gpd to the estimated design flow of 15 gpd per employee.

$$10 \text{ gallons per day per employee} \times 1.5 \text{ safety factor} = 15 \text{ gallons per day per employee}$$

It is estimated that once construction is completed, Cross Island Provisions will occupy both of the warehouse spaces and have 30 employees. Therefore, the total design flow can be calculated at 450 gallons per day.

$$15 \text{ gallons per day per employee} \times 30 \text{ employees} = 450 \text{ gallons per day}$$

Below is the full sanitary system design calculation.

DESIGN FLOW = 450 GPD (BASED ON WATER USAGE DATA)

PERCOLATION RATE = 1.0-10.0 MIN/IN

APPLICATION RATE = 1.5 GAL/SF/DAY

REQUIRED EFFECTIVE LEACHING AREA = $450/1.5 = 300$ SF

PROVIDED 1 - 60 LF ROWS OF GST 6212

EFFECTIVE LEACHING AREA PROVIDED =

$1 \times 60 \text{ LF} \times 10.0 \text{ LF/SF} = 600 \text{ SF}$

MINIMUM LEACHING SYSTEM SPREAD (MLSS)

DEPTH TO RESTRICTIVE LAYER = 38 INCHES

(BASED ON GROUND WATER MONITORING TP-10)

SLOPE= 1.0 %

HF= 36

FF= $450/300 = 1.5$

PF= 1

MLSS REQUIRED = $36 \times 1.5 \times 1 = 54$ LF

MLSS PROVIDED = 1 ROW X 60 = 60 LF

Appendix A
Design Computations

Water Quality Volume

WQV, Water Quality Volume (AC-FT)

RCV, Runoff Capture Volume (AC-FT)

R, Volumetric Runoff Coefficient

I, Percent Impervious Cover

A, Site Area (AC)

$$I = 69.02$$

$$R = 0.05 + 0.009(69) = 0.671$$

$$A = 1.84$$

$$\begin{aligned} \text{WQV} &= \frac{1" \times R \times A}{12} = \frac{1 \times 0.671 \times 1.84}{12} = 0.1029 \text{ AC-FT} \\ &= 4482.9 \text{ CF} \end{aligned}$$

WQV=4482.9 CF

Groundwater Recharge

GVR=Groundwater Recharge Volume (ac-ft)

D=Depth of Runoff to be Recharged (inches) (Table 7-4)

A=Site Area (acres)

I=Post Development Impervious (decimal)

net inches increase in site impervious for redevelopment

$$GRV = \frac{(D) (A) (I)}{12}$$

Table 7-4 Groundwater Recharge Depth		
NCRS Hydrologic Soil Groups	Average Annual Recharge	Groundwater Recharge Depth (D)
A	18 inches/year	0.4 inches
B	12 inches/year	0.25 inches
C	6 inches/year	0.1 inches
D	3 inches/year	0 inches (waived)

Existing Impervious	0.67
Proposed Impervious	1.27
Change In Impervious	0.6

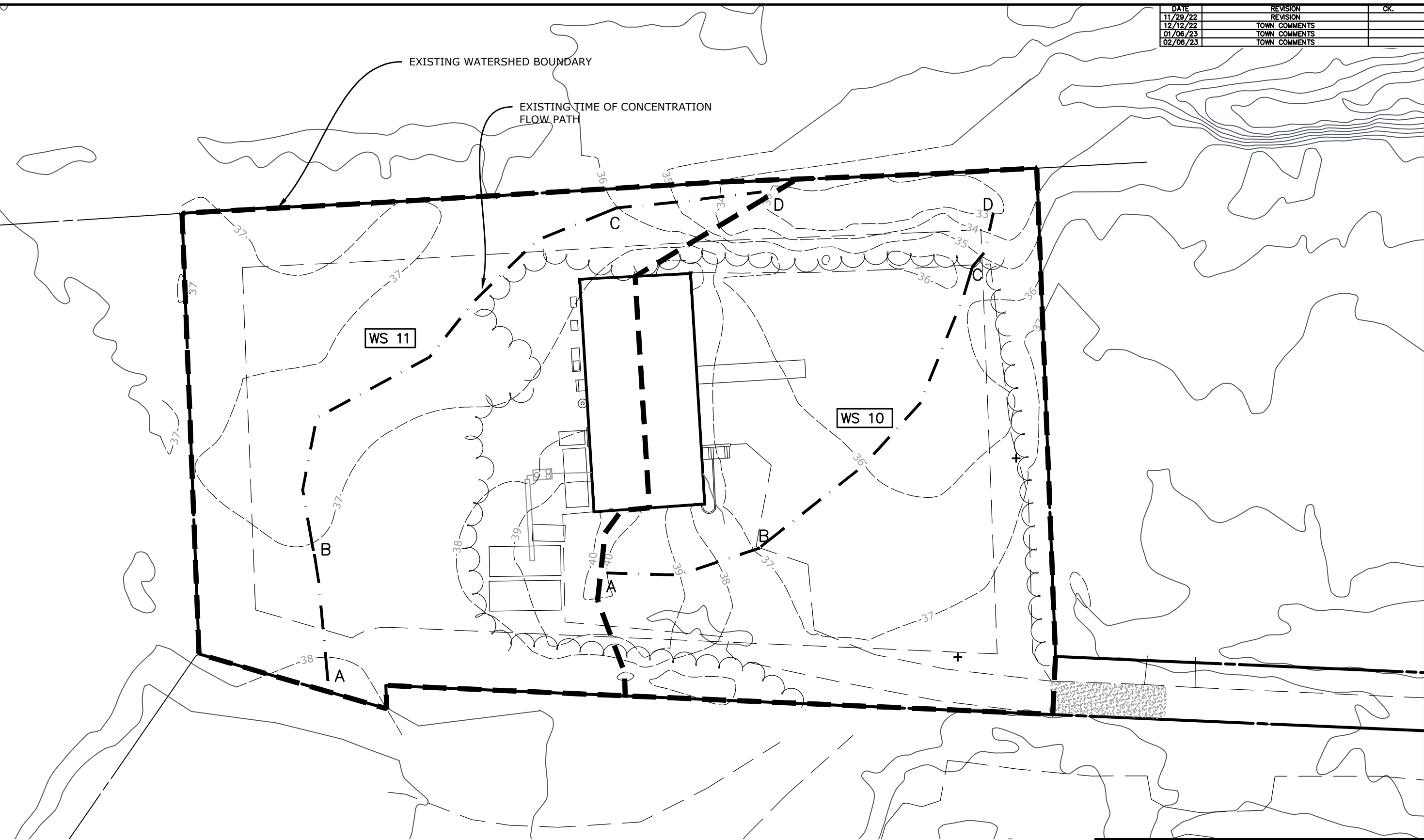
$$GRV = \frac{0.25 \quad 1.84 \quad 0.6}{12}$$

GRV=	0.023	ac-ft
	1001.9	cf

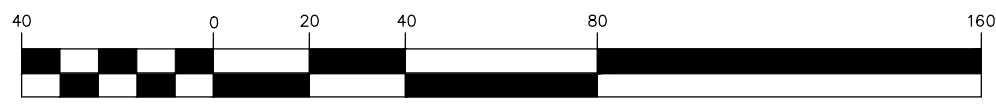
Appendix B
Hydrologic Model Input Data and Results

DATE	REVISION	CK.
11/29/22	REVISION	
12/12/22	TOWN COMMENTS	
01/06/23	TOWN COMMENTS	
02/06/23	TOWN COMMENTS	

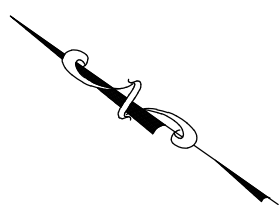
EXISTING WATERSHED BOUNDARY
 EXISTING TIME OF CONCENTRATION
 FLOW PATH




GRAPHIC SCALE



(IN FEET)
 1 inch = 40 ft.





DOANE ENGINEERING
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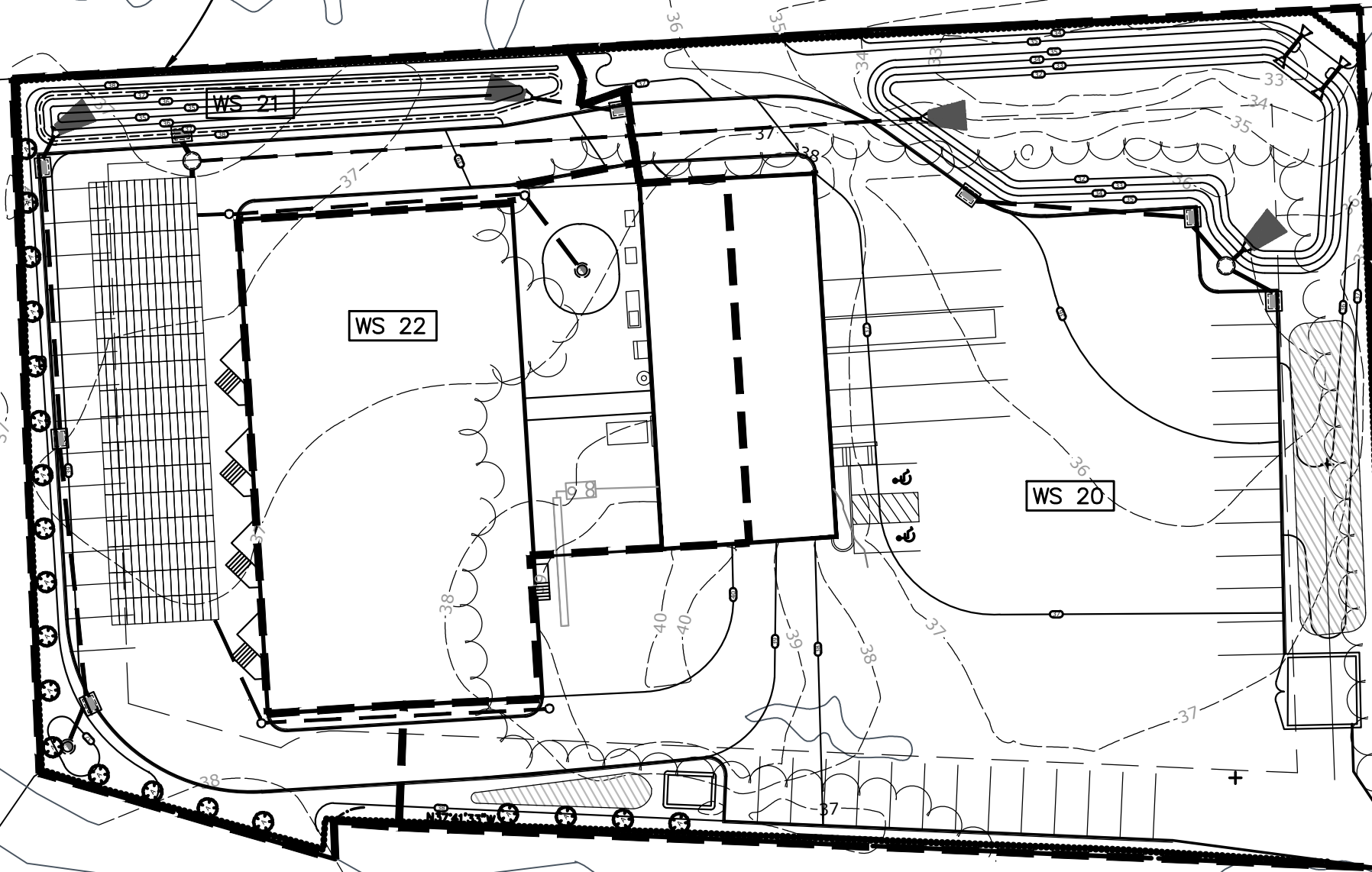
WATERSHED AREAS EXISTING CONDITIONS
 PREPARED FOR
PIAGE MANAGEMENT CORP
 #49 PLAINS ROAD , ESSEX , CONNECTICUT

SCALE: 1"=40'	DATE: 11/03/22	SHEET NO.: 1 OF 2	IDENT. NO.:
------------------	-------------------	----------------------	-------------

FILE: P:\DRAWINGS\TOWNS\ESSEX\PLAINS ROAD\PIAGENTM-BOB\Hydro Areas.dwg

DATE	REVISION	CK.
11/29/22	REVISION	
12/12/22	TOWN COMMENTS	
01/06/23	TOWN COMMENTS	
02/06/23	TOWN COMMENTS	

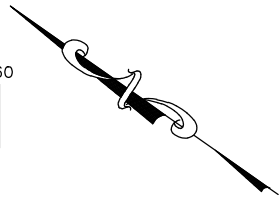
PROPOSED WATERSHED BOUNDARY




GRAPHIC SCALE



(IN FEET)
1 inch = 40 ft.





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WATERSHED AREAS PROPOSED CONDITIONS
PREPARED FOR
PIAGE MANAGEMENT CORP
#49 PLAINS ROAD , ESSEX , CONNECTICUT

SCALE: 1"=40'	DATE: 11/03/22	SHEET NO.: 2 OF 2	IDENT. NO.:
------------------	-------------------	----------------------	-------------

FILE: R:\DRAWINGS\TOWNS\ESSEX\PLAINS ROAD\PIAGENTM-BOB\Hydro Areas.dwg

Watershed Area's

Existing Watershed WS 10		
	SF	AC
Woods	9000	0.21
Grass	1200	0.03
Gravel	19300	0.44
Impervious	10000	0.23
Total	39500	0.91

Existing Watershed WS 11		
	SF	AC
Woods	30500	0.7
Grass	5600	0.13
Impervious	4500	0.1
Total	40600	0.93

Proposed Watershed WS 20		
		AC
Grass	14755	0.34
Impervious (Bituminous)	29400	0.67
Impervious (Building)	2375	0.05
Total	46530	1.07

Proposed Water Shed WS 21		
	SF	AC
Grass	6260	0.14
Impervious (Bituminous)	11200	0.26
Total	17460	0.4

Proposed Water Shed WS 22		
	SF	AC
Impervious (Building)	12500	0.29
Impervious (Bituminous)	210	0
Grass	3400	0.08
Total	16110	0.37

Time of Concentration (T_c) or Travel Time (T_t) Worksheet

Circle one: **Present** Developed
 Circle one: **T_c** T_t

Watershed: EX WS10
 Subwatershed: _____

Sheet flow (applicable to T_c only)

1. Surface description (Table 3-1)
2. Manning's roughness coeff. for sheet flow, n (Table 3-1)
3. Flow Length, L (< 300ft)
4. Two-year 24-hr rainfall, P_2
5. Land slope, s
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}(s^{0.4})}$

Segment ID	A-B				
	BIT				
	0.010				
ft.	65.0				
in.	3.44				
ft./ft.	0.040				
hr.	0.010	=		0.010	

Shallow concentrated flow (assume hyd. radius = depth of flow)

7. Surface description
8. Manning's roughness coeff., n
9. Paved or unpaved
10. Depth of flow, d (default values: d=.4 unpaved, d=.2 paved) ft.
11. Flow Length, L
12. Watercourse slope, s
13. Average velocity, $V = \frac{1.49}{n}(d^{2/3})(s^{1/2})$
14. $T_t = \frac{L}{3600 * V}$

Segment ID	B-C	C-D			
	BIT	WOODS			
	0.015	0.100			
	UNPVD	UNPVD			
	0.40	0.40			
ft.	160.0	25.0			
ft./ft.	0.005	0.100			
fps.	3.81	2.56			
hr.	0.012	+ 0.003			= 0.014

Channel flow

15. Channel Bottom width, b
16. Horizontal side slope component, z (z horiz:1 vert) ft.
17. Depth of flow, d
18. Cross sectional flow area, A (assume trapazoidal) ft.²
19. Wetted perimeter, P_w
20. Hydraulic Radius, $R = \frac{A}{P_w}$
21. Channel slope, s
22. Manning's roughness coeff., n
23. $V = \frac{1.49}{n}(R^{2/3})(s^{1/2})$
24. Flow length, L
25. $T_t = \frac{L}{3600 * V}$
26. Watershed or subarea T_c or T_t (add T_t in steps 6, 14 & 25)

Segment ID					
ft.					
ft.					
ft. ²					
ft.					
ft.					
ft./ft.					
fps.					
ft.					
hr.					= 0.000
					0.024

Time of Concentration (T_c) or Travel Time (T_t) Worksheet

Circle one: **Present** Developed
 Circle one: **T_c** T_t

Watershed: EX WS11
 Subwatershed: _____

Sheet flow (applicable to T_c only)

1. Surface description (Table 3-1)
2. Manning's roughness coeff. for sheet flow, n (Table 3-1)
3. Flow Length, L (< 300ft)
4. Two-year 24-hr rainfall, P₂
5. Land slope, s
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} (s^{0.4})}$

Segment ID	A-B	
	WOODS	
	0.400	
ft.	50.0	
in.	3.44	
ft./ft.	0.020	
hr.	0.198	= 0.198

Shallow concentrated flow (assume hyd. radius = depth of flow)

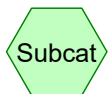
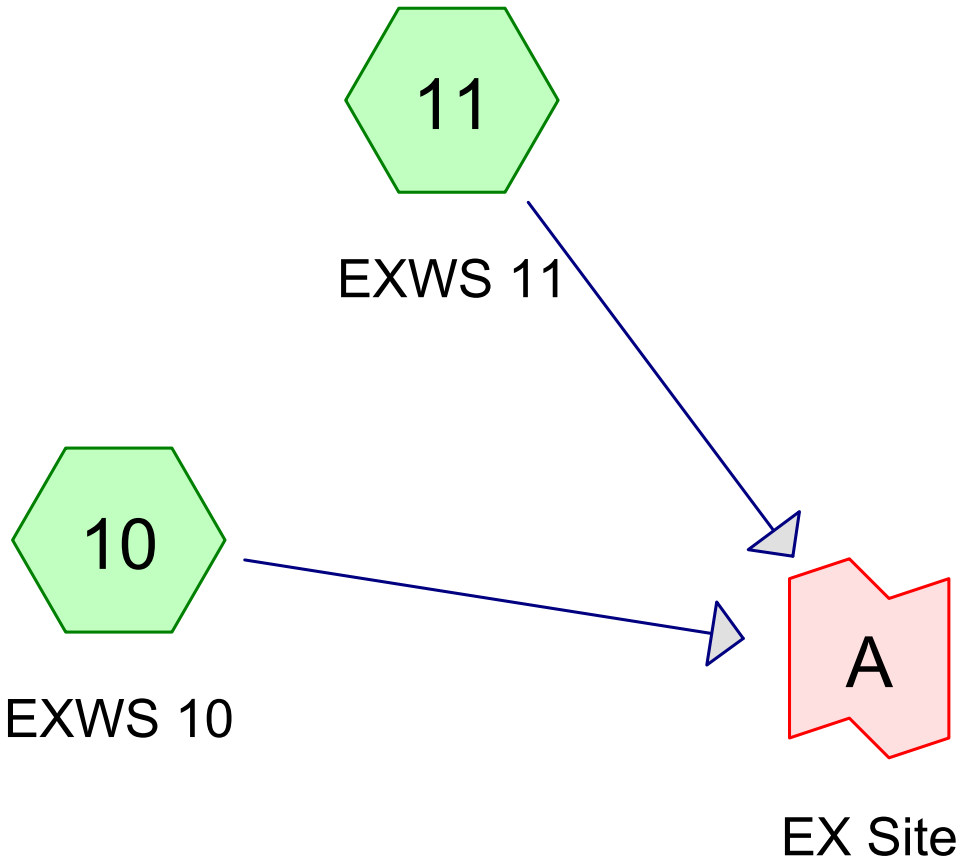
7. Surface description
8. Manning's roughness coeff., n
9. Paved or unpaved
10. Depth of flow, d (default values: d=.4 unpaved, d=.2 paved) ft.
11. Flow Length, L
12. Watercourse slope, s
13. Average velocity, $V = \frac{1.49}{n} (d^{2/3}) (s^{1/2})$
14. $T_t = \frac{L}{3600 * V}$

Segment ID	B-C	C-D		
	WOODS	WOODS		
	0.400	0.400		
	UNPVD	UNPVD		
ft.	0.40	0.40		
ft./ft.	0.010	0.050		
fps.	0.20	0.45		
hr.	0.299	+ 0.037		= 0.336

Channel flow

15. Channel Bottom width, b
16. Horizontal side slope component, z (z horiz:1 vert) ft.
17. Depth of flow, d
18. Cross sectional flow area, A (assume trapazoidal) ft.²
19. Wetted perimeter, P_w
20. Hydraulic Radius, $R = \frac{A}{P_w}$
21. Channel slope, s
22. Manning's roughness coeff., n
23. $V = \frac{1.49}{n} (R^{2/3}) (s^{1/2})$
24. Flow length, L
25. $T_t = \frac{L}{3600 * V}$
26. Watershed or subarea T_c or T_t (add T_t in steps 6, 14 & 25)

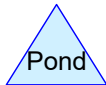
Segment ID				
ft.				
ft.				
ft. ²				
ft.				
ft.				
ft./ft.				
fps.				
ft.				
hr.				= 0.000
hr.				0.535



Subcat



Reach



Pond



Link

49 Plains Road Existing

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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-yr	CT-49 Plains Road Essex 24-hr S1	1-yr	Default	24.00	1	2.85	2
2	2-yr	CT-49 Plains Road Essex 24-hr S1	2-yr	Default	24.00	1	3.44	2
3	5-yr	CT-49 Plains Road Essex 24-hr S1	5-yr	Default	24.00	1	4.40	2
4	10-yr	CT-49 Plains Road Essex 24-hr S1	10-yr	Default	24.00	1	5.20	2
5	25-yr	CT-49 Plains Road Essex 24-hr S1	25-yr	Default	24.00	1	6.31	2
6	50-yr	CT-49 Plains Road Essex 24-hr S1	50-yr	Default	24.00	1	7.13	2
7	100-yr	CT-49 Plains Road Essex 24-hr S1	100-yr	Default	24.00	1	8.01	2

49 Plains Road Existing

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.156	61	>75% Grass cover, Good, HSG B (10, 11)
0.443	96	Gravel surface, HSG B (10)
0.333	98	Impervious (10, 11)
0.907	55	Woods, Good, HSG B (10, 11)
1.839	73	TOTAL AREA

49 Plains Road Existing

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.156	0.000	0.000	0.000	0.156	>75% Grass cover, Good	10, 11
0.000	0.443	0.000	0.000	0.000	0.443	Gravel surface	10
0.000	0.000	0.000	0.000	0.333	0.333	Impervious	10, 11
0.000	0.907	0.000	0.000	0.000	0.907	Woods, Good	10, 11
0.000	1.506	0.000	0.000	0.333	1.839	TOTAL AREA	

49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10: EXWS 10

Runoff Area=39,500 sf 25.32% Impervious Runoff Depth>1.53"
Tc=6.0 min CN=86 Runoff=1.76 cfs 0.116 af

Subcatchment 11: EXWS 11

Runoff Area=40,600 sf 11.08% Impervious Runoff Depth>0.30"
Tc=32.1 min CN=61 Runoff=0.08 cfs 0.024 af

Link A: EX Site

Inflow=1.76 cfs 0.140 af
Primary=1.76 cfs 0.140 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.140 af Average Runoff Depth = 0.91"
81.90% Pervious = 1.506 ac 18.10% Impervious = 0.333 ac

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CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"

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Summary for Subcatchment 10: EXWS 10

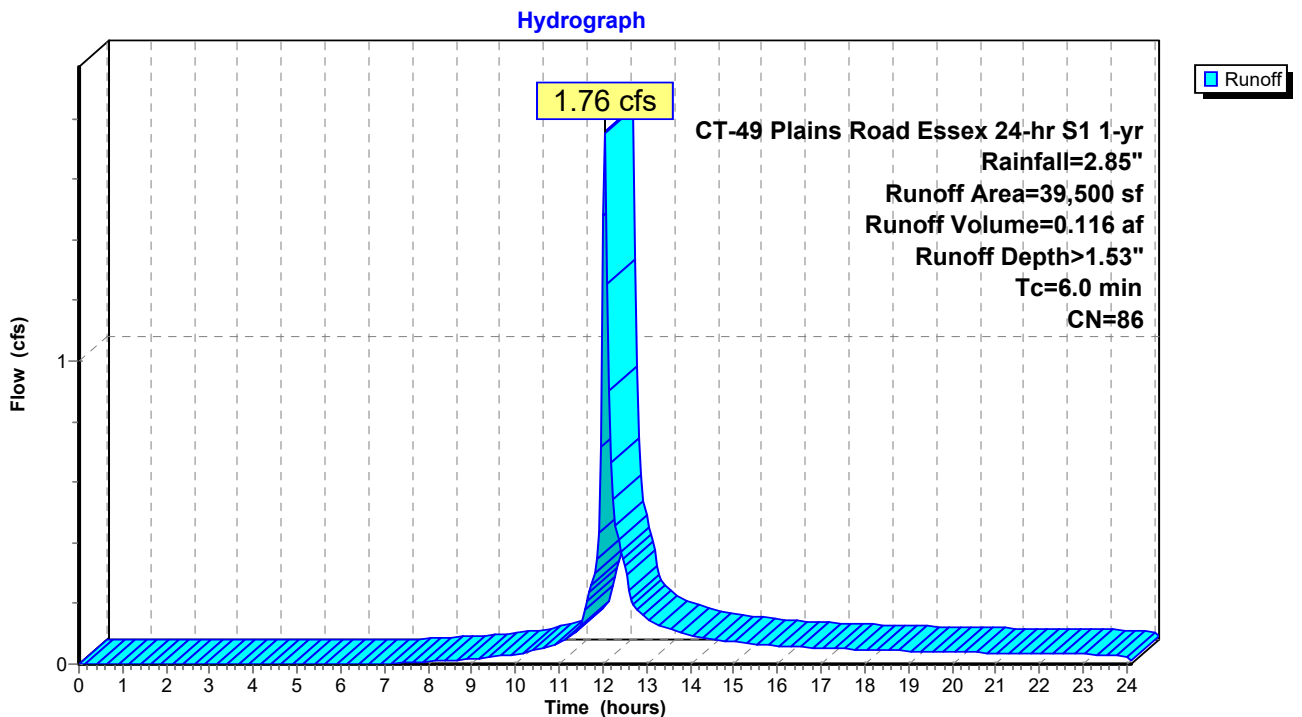
Runoff = 1.76 cfs @ 12.04 hrs, Volume= 0.116 af, Depth> 1.53"
Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"

Area (sf)	CN	Description
9,000	55	Woods, Good, HSG B
1,200	61	>75% Grass cover, Good, HSG B
19,300	96	Gravel surface, HSG B
* 10,000	98	Impervious
39,500	86	Weighted Average
29,500		74.68% Pervious Area
10,000		25.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, MIN TR-55 TC 6.0 MIN

Subcatchment 10: EXWS 10



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CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"

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Summary for Subcatchment 11: EXWS 11

Runoff = 0.08 cfs @ 12.60 hrs, Volume= 0.024 af, Depth> 0.30"
 Routed to Link A : EX Site

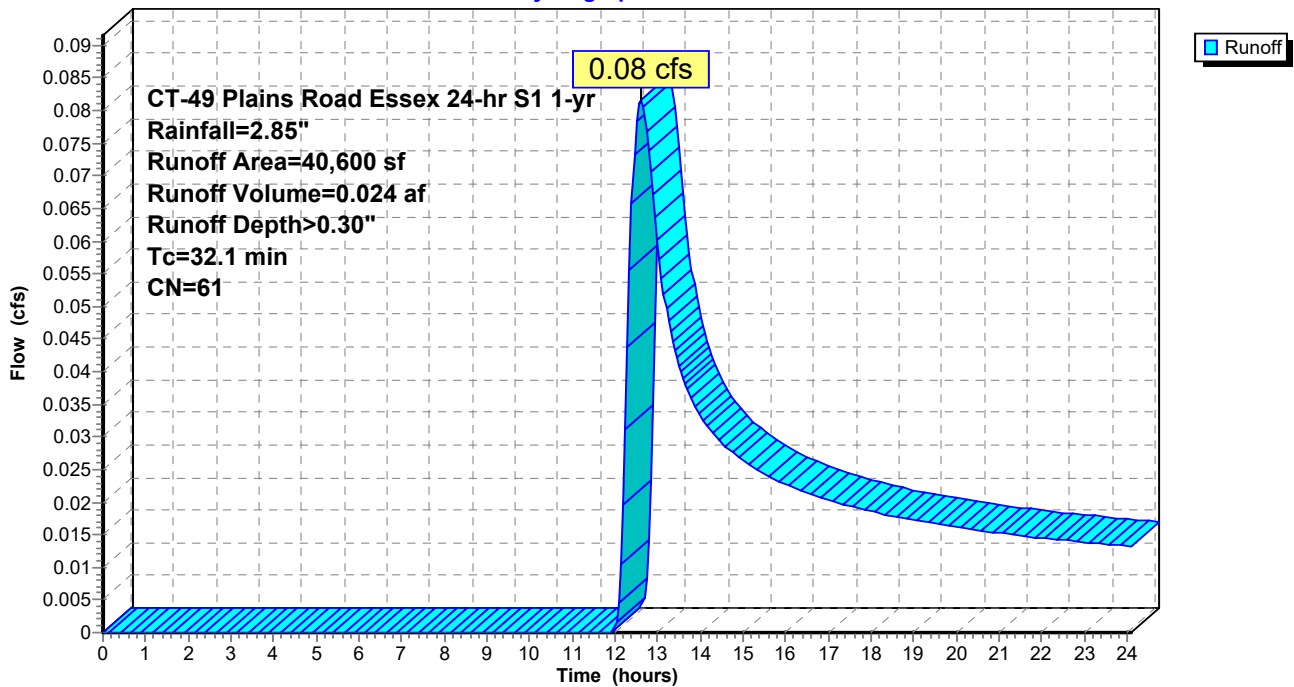
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"

Area (sf)	CN	Description
30,500	55	Woods, Good, HSG B
5,600	61	>75% Grass cover, Good, HSG B
* 4,500	98	Impervious
40,600	61	Weighted Average
36,100		88.92% Pervious Area
4,500		11.08% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.1					Direct Entry, See Worksheet

Subcatchment 11: EXWS 11

Hydrograph



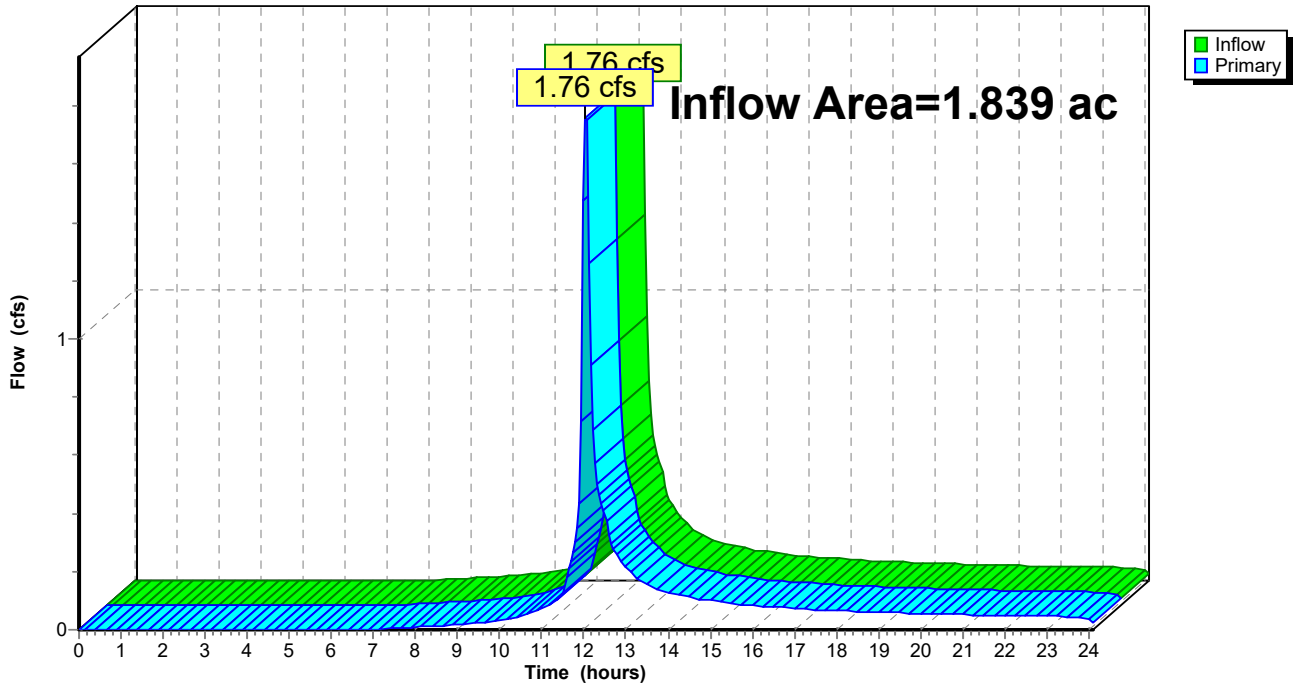
Summary for Link A: EX Site

Inflow Area = 1.839 ac, 18.10% Impervious, Inflow Depth > 0.91" for 1-yr event
Inflow = 1.76 cfs @ 12.04 hrs, Volume= 0.140 af
Primary = 1.76 cfs @ 12.04 hrs, Volume= 0.140 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs

Link A: EX Site

Hydrograph



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10: EXWS 10

Runoff Area=39,500 sf 25.32% Impervious Runoff Depth>2.05"
Tc=6.0 min CN=86 Runoff=2.34 cfs 0.155 af

Subcatchment 11: EXWS 11

Runoff Area=40,600 sf 11.08% Impervious Runoff Depth>0.54"
Tc=32.1 min CN=61 Runoff=0.20 cfs 0.042 af

Link A: EX Site

Inflow=2.35 cfs 0.196 af
Primary=2.35 cfs 0.196 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.196 af Average Runoff Depth = 1.28"
81.90% Pervious = 1.506 ac 18.10% Impervious = 0.333 ac

49 Plains Road Existing

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CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"

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Summary for Subcatchment 10: EXWS 10

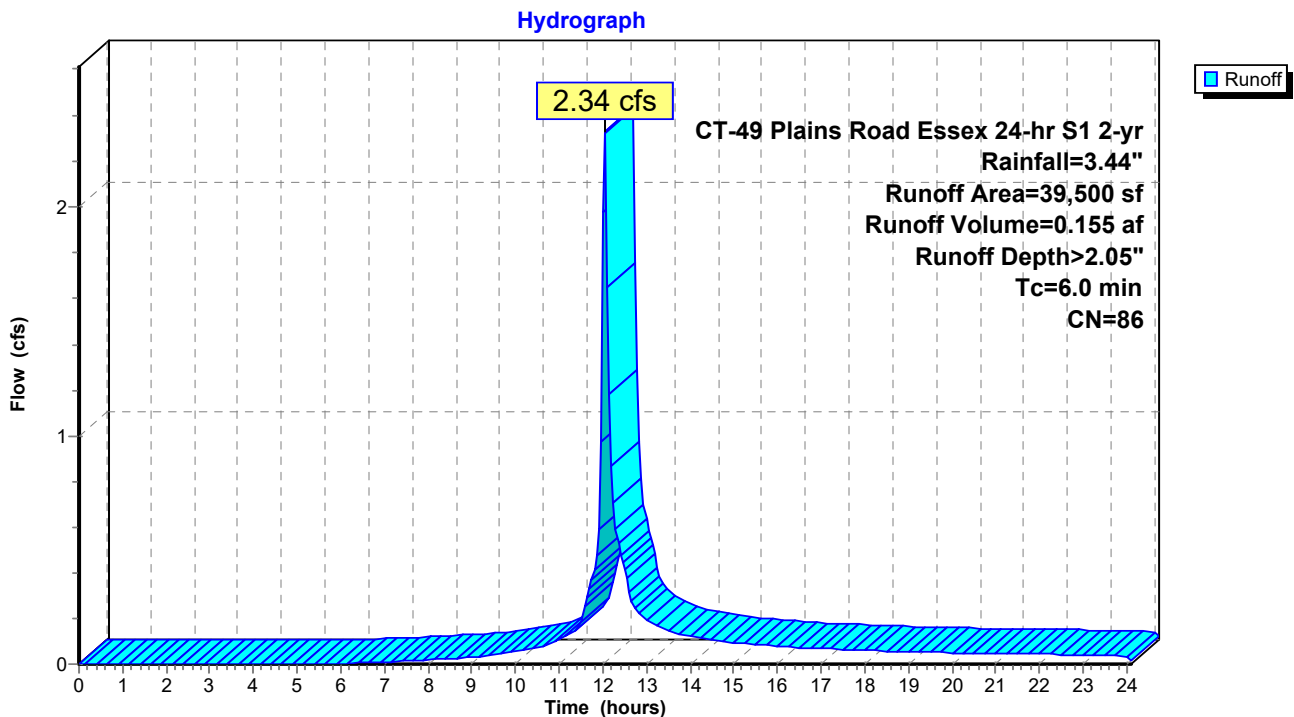
Runoff = 2.34 cfs @ 12.04 hrs, Volume= 0.155 af, Depth> 2.05"
Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"

Area (sf)	CN	Description
9,000	55	Woods, Good, HSG B
1,200	61	>75% Grass cover, Good, HSG B
19,300	96	Gravel surface, HSG B
* 10,000	98	Impervious
39,500	86	Weighted Average
29,500		74.68% Pervious Area
10,000		25.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, MIN TR-55 TC 6.0 MIN

Subcatchment 10: EXWS 10



49 Plains Road Existing

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CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"

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Summary for Subcatchment 11: EXWS 11

Runoff = 0.20 cfs @ 12.49 hrs, Volume= 0.042 af, Depth> 0.54"
 Routed to Link A : EX Site

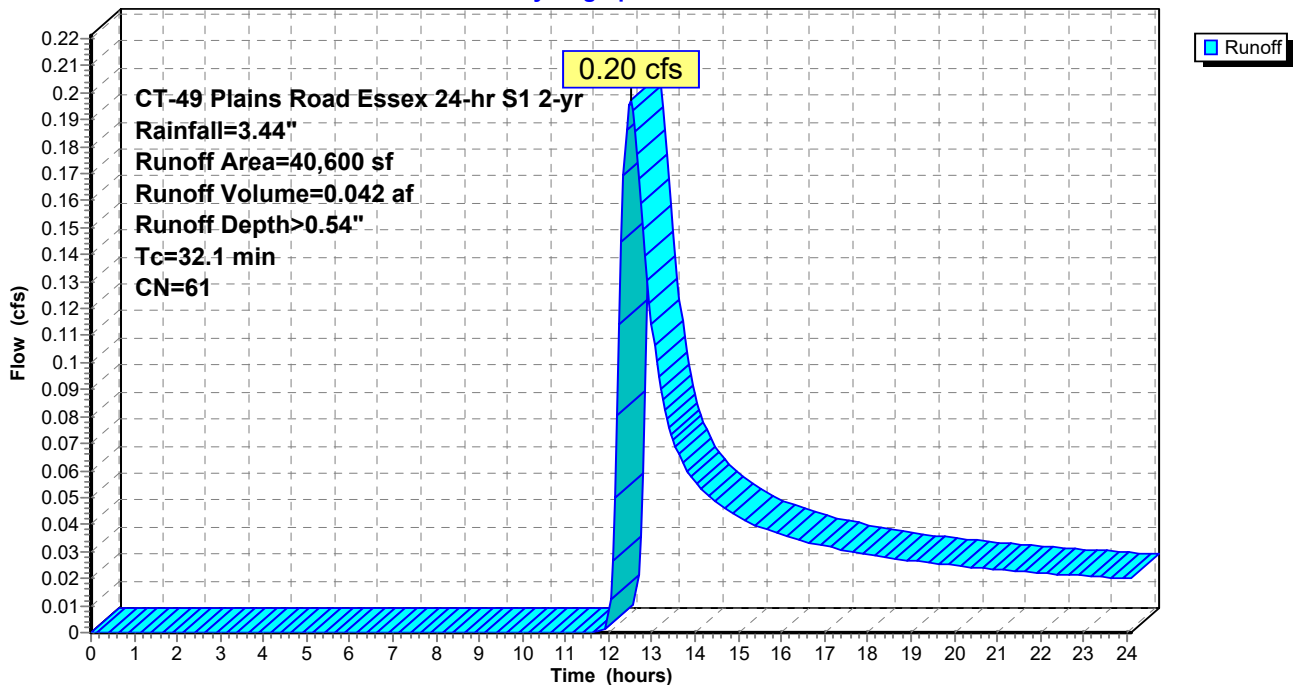
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"

Area (sf)	CN	Description
30,500	55	Woods, Good, HSG B
5,600	61	>75% Grass cover, Good, HSG B
* 4,500	98	Impervious
40,600	61	Weighted Average
36,100		88.92% Pervious Area
4,500		11.08% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.1					Direct Entry, See Worksheet

Subcatchment 11: EXWS 11

Hydrograph



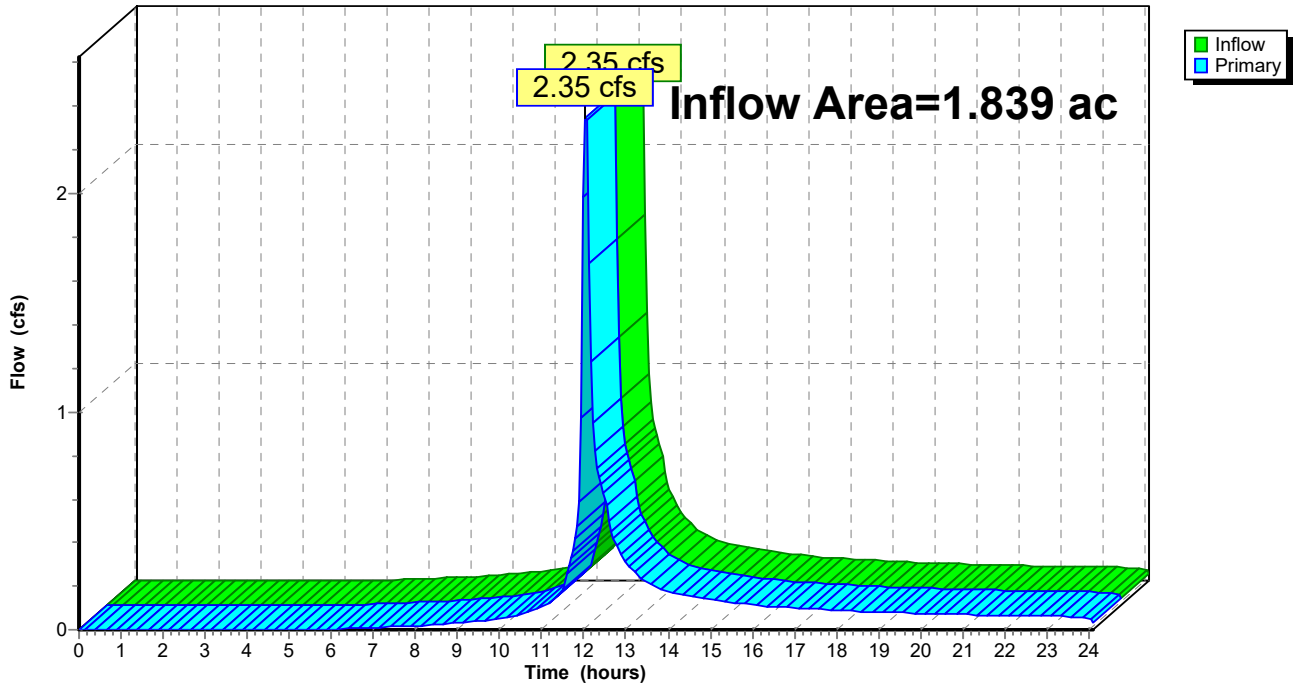
Summary for Link A: EX Site

Inflow Area = 1.839 ac, 18.10% Impervious, Inflow Depth > 1.28" for 2-yr event
Inflow = 2.35 cfs @ 12.04 hrs, Volume= 0.196 af
Primary = 2.35 cfs @ 12.04 hrs, Volume= 0.196 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs

Link A: EX Site

Hydrograph



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10: EXWS 10

Runoff Area=39,500 sf 25.32% Impervious Runoff Depth>2.91"
Tc=6.0 min CN=86 Runoff=3.29 cfs 0.220 af

Subcatchment 11: EXWS 11

Runoff Area=40,600 sf 11.08% Impervious Runoff Depth>1.01"
Tc=32.1 min CN=61 Runoff=0.46 cfs 0.079 af

Link A: EX Site

Inflow=3.37 cfs 0.299 af
Primary=3.37 cfs 0.299 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.299 af Average Runoff Depth = 1.95"
81.90% Pervious = 1.506 ac 18.10% Impervious = 0.333 ac

49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"

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Summary for Subcatchment 10: EXWS 10

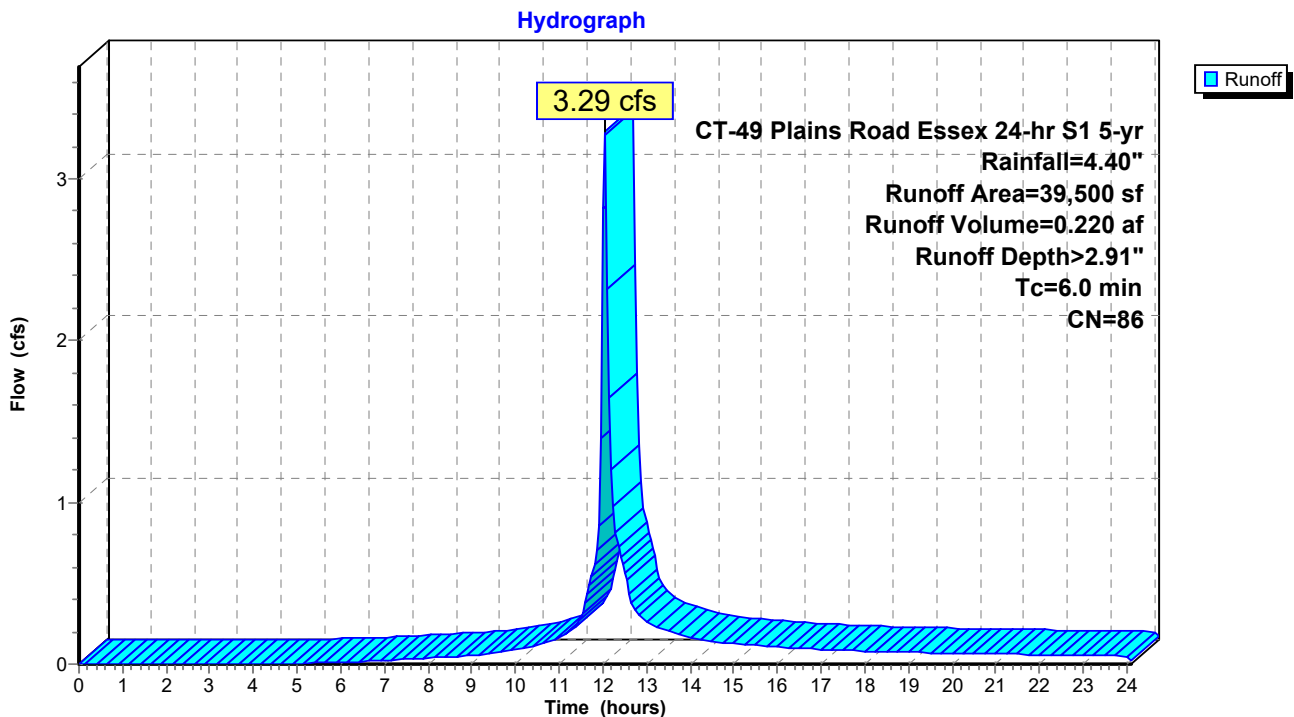
Runoff = 3.29 cfs @ 12.04 hrs, Volume= 0.220 af, Depth> 2.91"
 Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"

Area (sf)	CN	Description
9,000	55	Woods, Good, HSG B
1,200	61	>75% Grass cover, Good, HSG B
19,300	96	Gravel surface, HSG B
* 10,000	98	Impervious
39,500	86	Weighted Average
29,500		74.68% Pervious Area
10,000		25.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, MIN TR-55 TC 6.0 MIN

Subcatchment 10: EXWS 10



49 Plains Road Existing

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CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"

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Summary for Subcatchment 11: EXWS 11

Runoff = 0.46 cfs @ 12.44 hrs, Volume= 0.079 af, Depth> 1.01"
Routed to Link A : EX Site

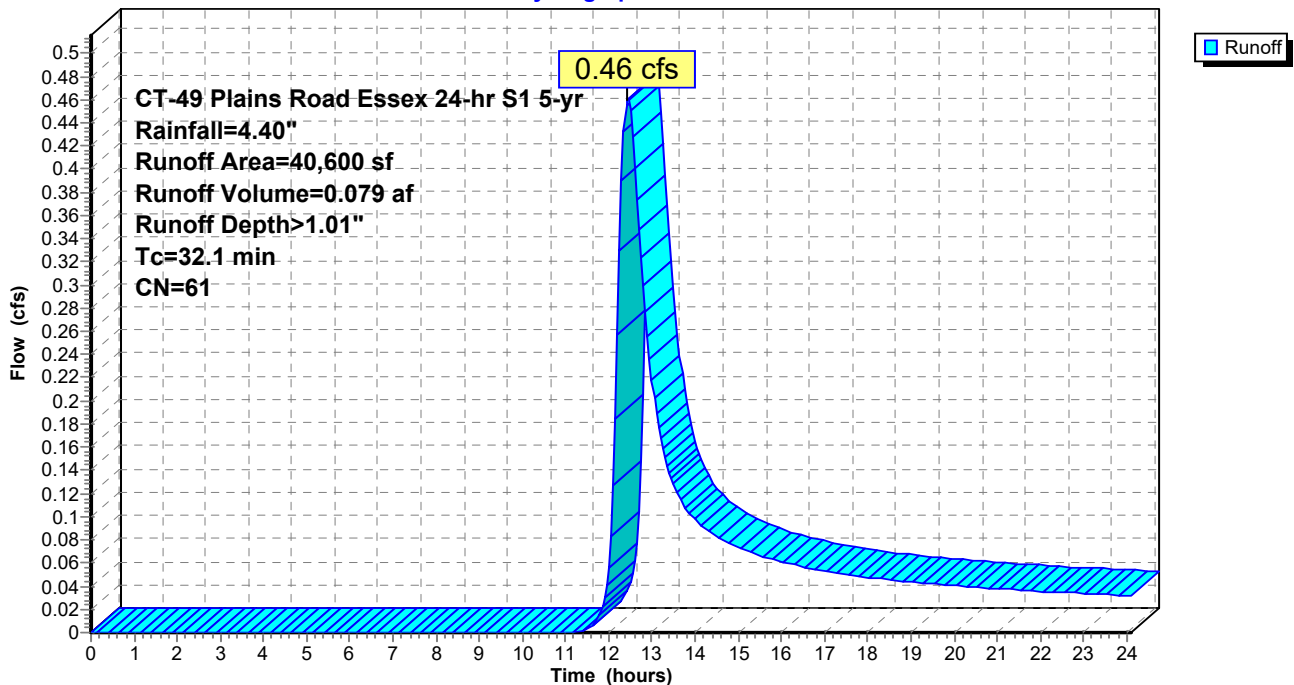
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"

Area (sf)	CN	Description
30,500	55	Woods, Good, HSG B
5,600	61	>75% Grass cover, Good, HSG B
* 4,500	98	Impervious
40,600	61	Weighted Average
36,100		88.92% Pervious Area
4,500		11.08% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.1					Direct Entry, See Worksheet

Subcatchment 11: EXWS 11

Hydrograph



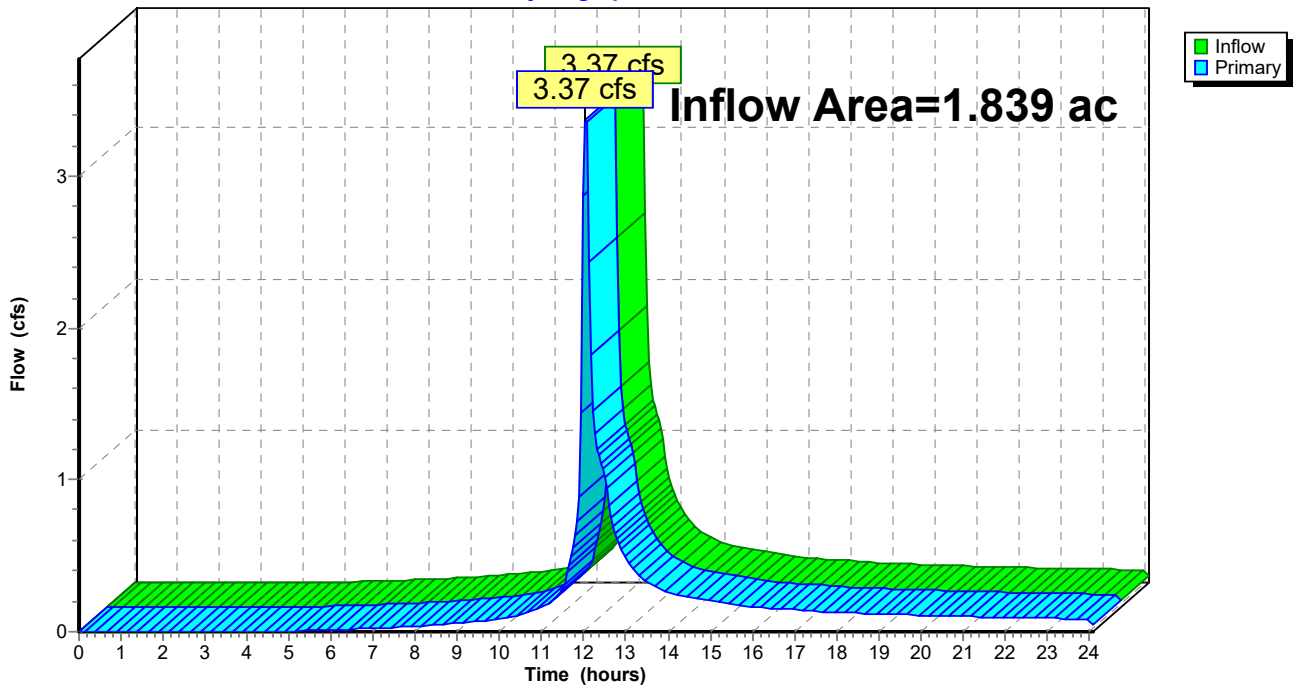
Summary for Link A: EX Site

Inflow Area = 1.839 ac, 18.10% Impervious, Inflow Depth > 1.95" for 5-yr event
Inflow = 3.37 cfs @ 12.04 hrs, Volume= 0.299 af
Primary = 3.37 cfs @ 12.04 hrs, Volume= 0.299 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs

Link A: EX Site

Hydrograph



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10: EXWS 10

Runoff Area=39,500 sf 25.32% Impervious Runoff Depth>3.65"
Tc=6.0 min CN=86 Runoff=4.09 cfs 0.276 af

Subcatchment 11: EXWS 11

Runoff Area=40,600 sf 11.08% Impervious Runoff Depth>1.47"
Tc=32.1 min CN=61 Runoff=0.72 cfs 0.114 af

Link A: EX Site

Inflow=4.25 cfs 0.391 af
Primary=4.25 cfs 0.391 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.391 af Average Runoff Depth = 2.55"
81.90% Pervious = 1.506 ac 18.10% Impervious = 0.333 ac

49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

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Summary for Subcatchment 10: EXWS 10

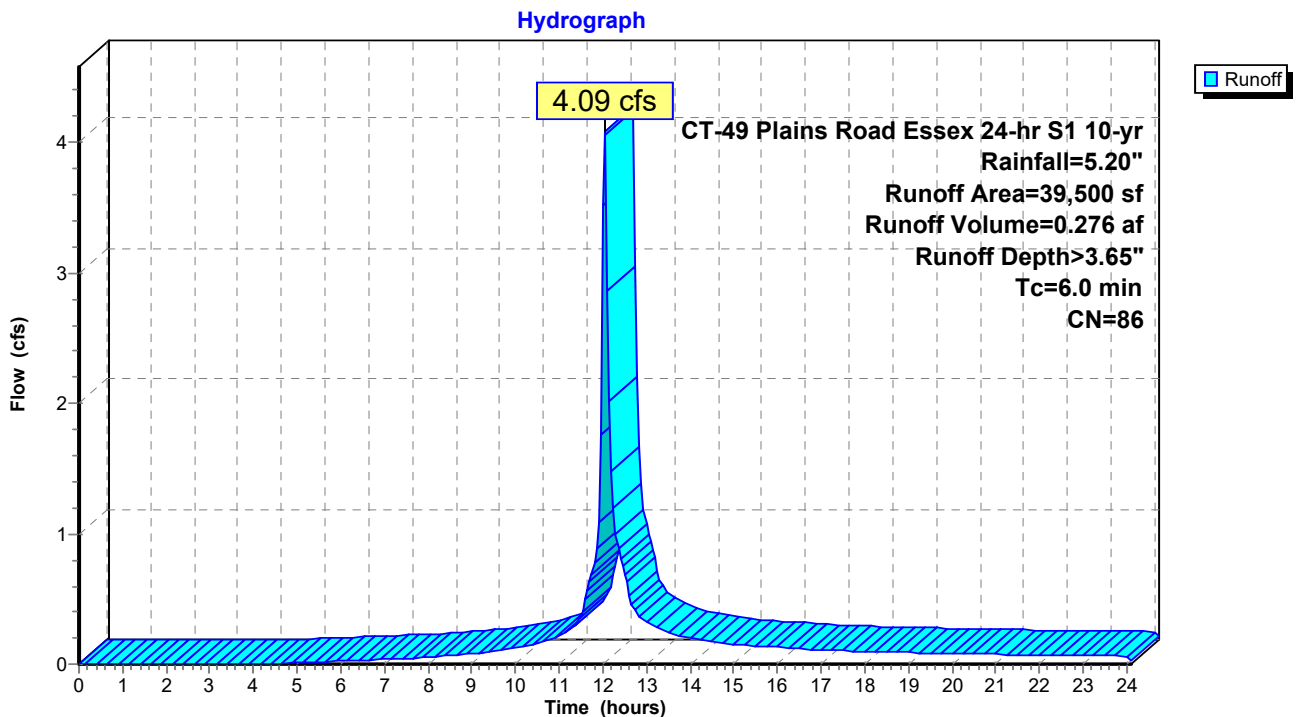
Runoff = 4.09 cfs @ 12.04 hrs, Volume= 0.276 af, Depth> 3.65"
 Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

Area (sf)	CN	Description
9,000	55	Woods, Good, HSG B
1,200	61	>75% Grass cover, Good, HSG B
19,300	96	Gravel surface, HSG B
* 10,000	98	Impervious
39,500	86	Weighted Average
29,500		74.68% Pervious Area
10,000		25.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, MIN TR-55 TC 6.0 MIN

Subcatchment 10: EXWS 10



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

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Summary for Subcatchment 11: EXWS 11

Runoff = 0.72 cfs @ 12.42 hrs, Volume= 0.114 af, Depth> 1.47"
Routed to Link A : EX Site

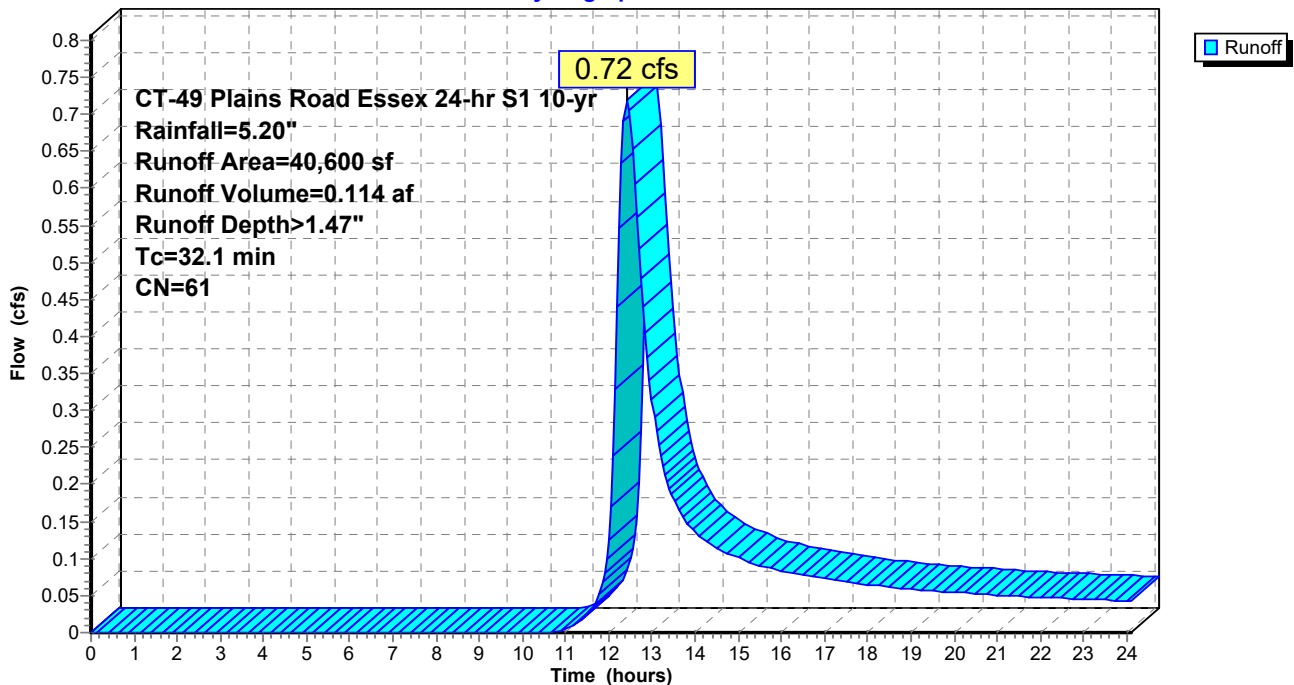
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

Area (sf)	CN	Description
30,500	55	Woods, Good, HSG B
5,600	61	>75% Grass cover, Good, HSG B
* 4,500	98	Impervious
40,600	61	Weighted Average
36,100		88.92% Pervious Area
4,500		11.08% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.1					Direct Entry, See Worksheet

Subcatchment 11: EXWS 11

Hydrograph



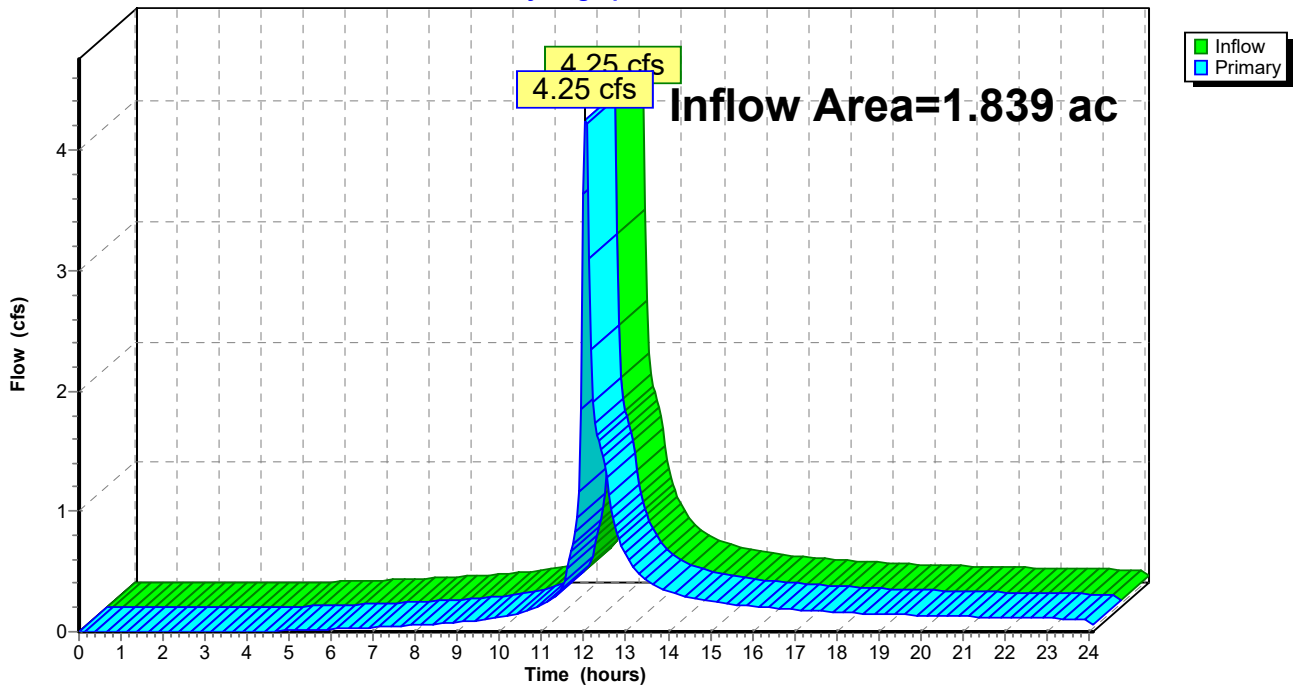
Summary for Link A: EX Site

Inflow Area = 1.839 ac, 18.10% Impervious, Inflow Depth > 2.55" for 10-yr event
Inflow = 4.25 cfs @ 12.04 hrs, Volume= 0.391 af
Primary = 4.25 cfs @ 12.04 hrs, Volume= 0.391 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs

Link A: EX Site

Hydrograph



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10: EXWS 10

Runoff Area=39,500 sf 25.32% Impervious Runoff Depth>4.70"
Tc=6.0 min CN=86 Runoff=5.19 cfs 0.355 af

Subcatchment 11: EXWS 11

Runoff Area=40,600 sf 11.08% Impervious Runoff Depth>2.19"
Tc=32.1 min CN=61 Runoff=1.12 cfs 0.170 af

Link A: EX Site

Inflow=5.49 cfs 0.526 af
Primary=5.49 cfs 0.526 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.526 af Average Runoff Depth = 3.43"
81.90% Pervious = 1.506 ac 18.10% Impervious = 0.333 ac

49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

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Summary for Subcatchment 10: EXWS 10

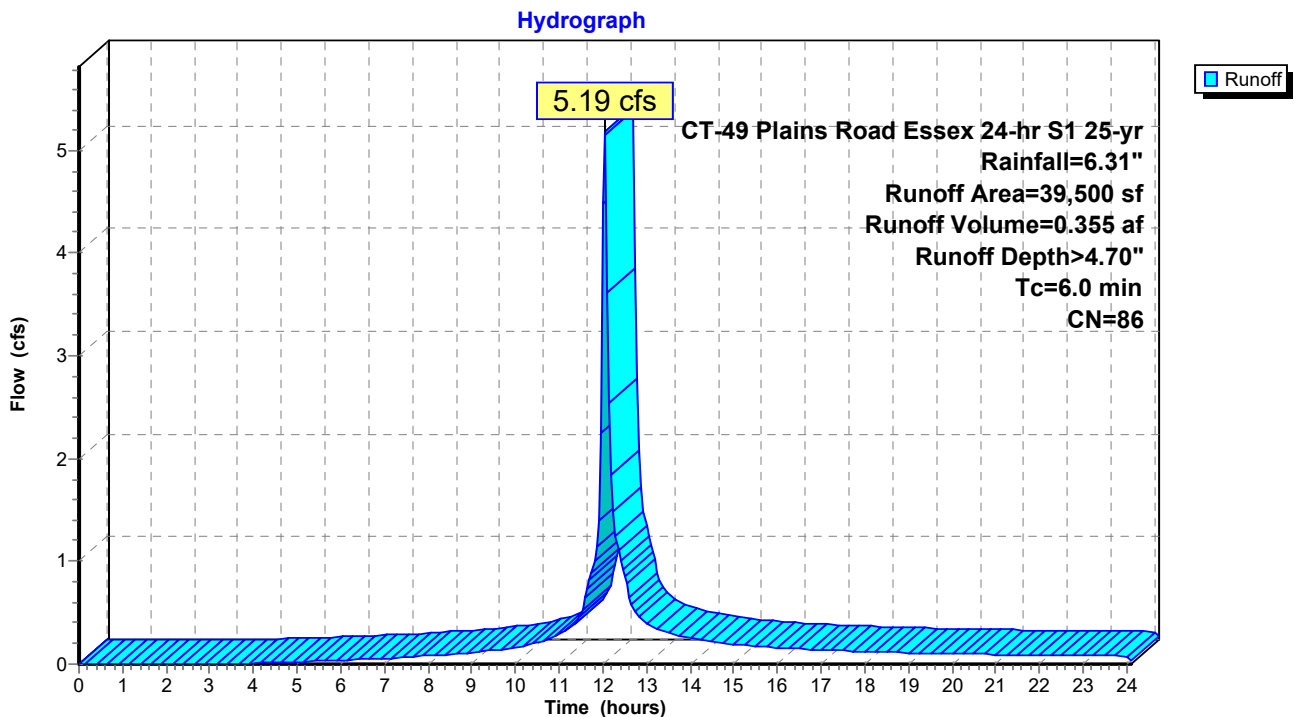
Runoff = 5.19 cfs @ 12.04 hrs, Volume= 0.355 af, Depth> 4.70"
 Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

Area (sf)	CN	Description
9,000	55	Woods, Good, HSG B
1,200	61	>75% Grass cover, Good, HSG B
19,300	96	Gravel surface, HSG B
* 10,000	98	Impervious
39,500	86	Weighted Average
29,500		74.68% Pervious Area
10,000		25.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, MIN TR-55 TC 6.0 MIN

Subcatchment 10: EXWS 10



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

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Summary for Subcatchment 11: EXWS 11

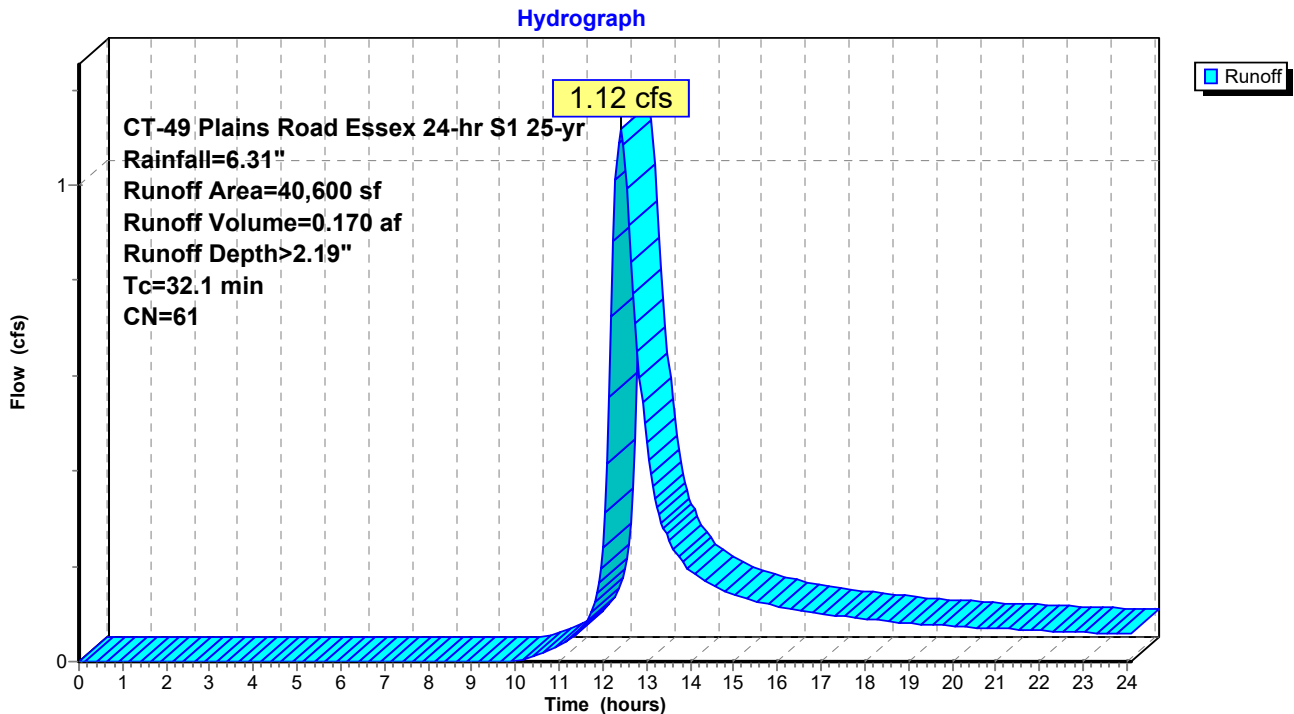
Runoff = 1.12 cfs @ 12.41 hrs, Volume= 0.170 af, Depth> 2.19"
Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

Area (sf)	CN	Description
30,500	55	Woods, Good, HSG B
5,600	61	>75% Grass cover, Good, HSG B
* 4,500	98	Impervious
40,600	61	Weighted Average
36,100		88.92% Pervious Area
4,500		11.08% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.1					Direct Entry, See Worksheet

Subcatchment 11: EXWS 11



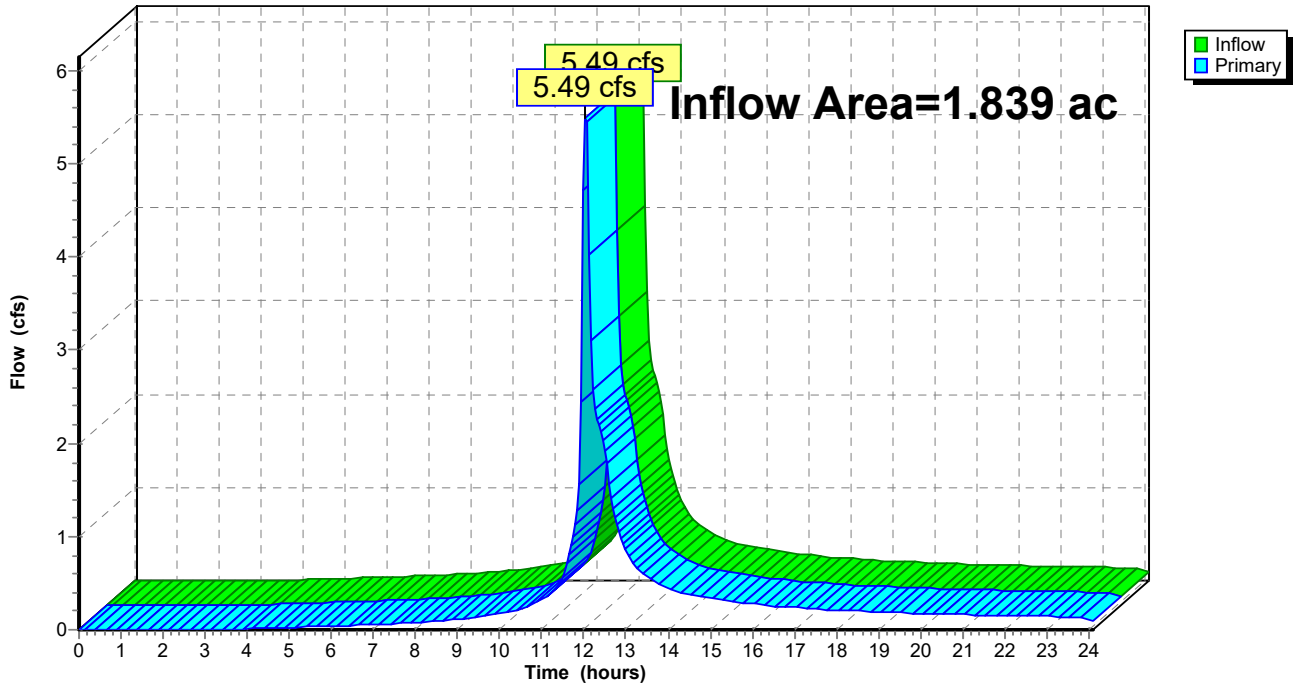
Summary for Link A: EX Site

Inflow Area = 1.839 ac, 18.10% Impervious, Inflow Depth > 3.43" for 25-yr event
Inflow = 5.49 cfs @ 12.04 hrs, Volume= 0.526 af
Primary = 5.49 cfs @ 12.04 hrs, Volume= 0.526 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs

Link A: EX Site

Hydrograph



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 50-yr Rainfall=7.13"

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10: EXWS 10

Runoff Area=39,500 sf 25.32% Impervious Runoff Depth>5.49"
Tc=6.0 min CN=86 Runoff=6.02 cfs 0.415 af

Subcatchment 11: EXWS 11

Runoff Area=40,600 sf 11.08% Impervious Runoff Depth>2.77"
Tc=32.1 min CN=61 Runoff=1.45 cfs 0.215 af

Link A: EX Site

Inflow=6.42 cfs 0.630 af
Primary=6.42 cfs 0.630 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.630 af Average Runoff Depth = 4.11"
81.90% Pervious = 1.506 ac 18.10% Impervious = 0.333 ac

49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 50-yr Rainfall=7.13"

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Summary for Subcatchment 10: EXWS 10

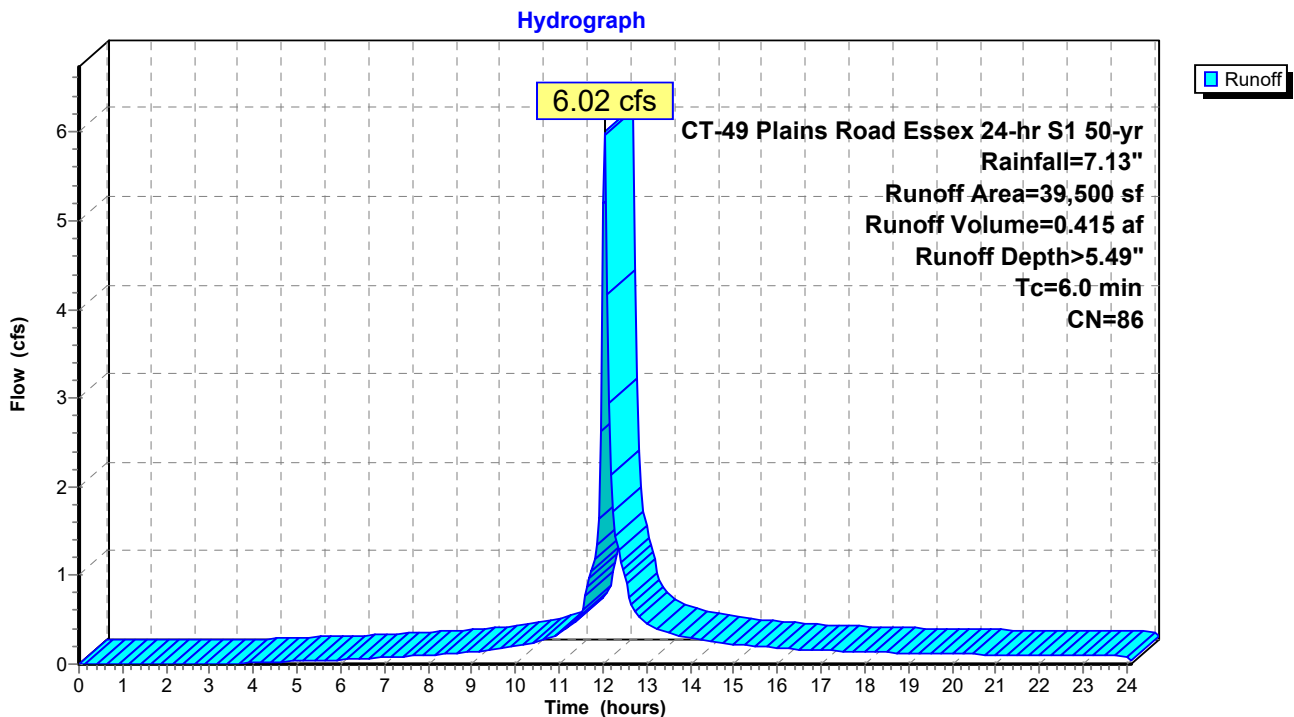
Runoff = 6.02 cfs @ 12.04 hrs, Volume= 0.415 af, Depth> 5.49"
 Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 50-yr Rainfall=7.13"

Area (sf)	CN	Description
9,000	55	Woods, Good, HSG B
1,200	61	>75% Grass cover, Good, HSG B
19,300	96	Gravel surface, HSG B
* 10,000	98	Impervious
39,500	86	Weighted Average
29,500		74.68% Pervious Area
10,000		25.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, MIN TR-55 TC 6.0 MIN

Subcatchment 10: EXWS 10



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 50-yr Rainfall=7.13"

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Summary for Subcatchment 11: EXWS 11

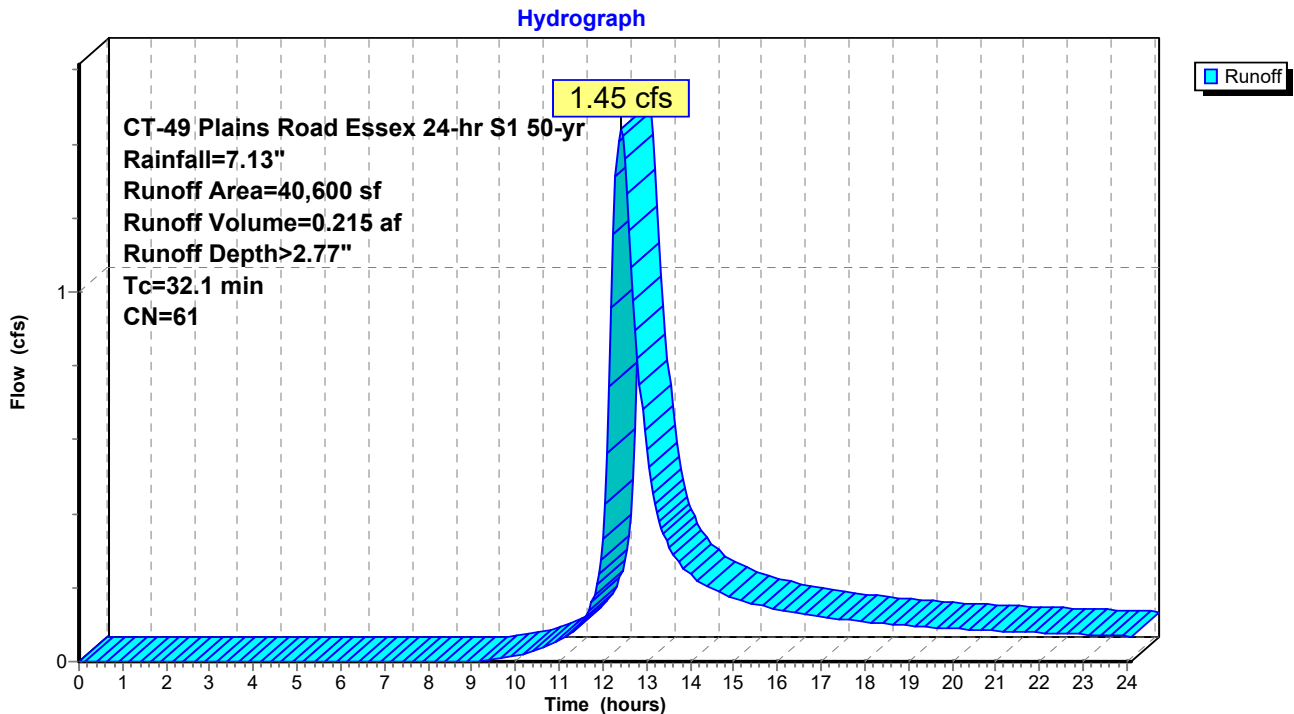
Runoff = 1.45 cfs @ 12.41 hrs, Volume= 0.215 af, Depth> 2.77"
Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
CT-49 Plains Road Essex 24-hr S1 50-yr Rainfall=7.13"

Area (sf)	CN	Description
30,500	55	Woods, Good, HSG B
5,600	61	>75% Grass cover, Good, HSG B
* 4,500	98	Impervious
40,600	61	Weighted Average
36,100		88.92% Pervious Area
4,500		11.08% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.1					Direct Entry, See Worksheet

Subcatchment 11: EXWS 11



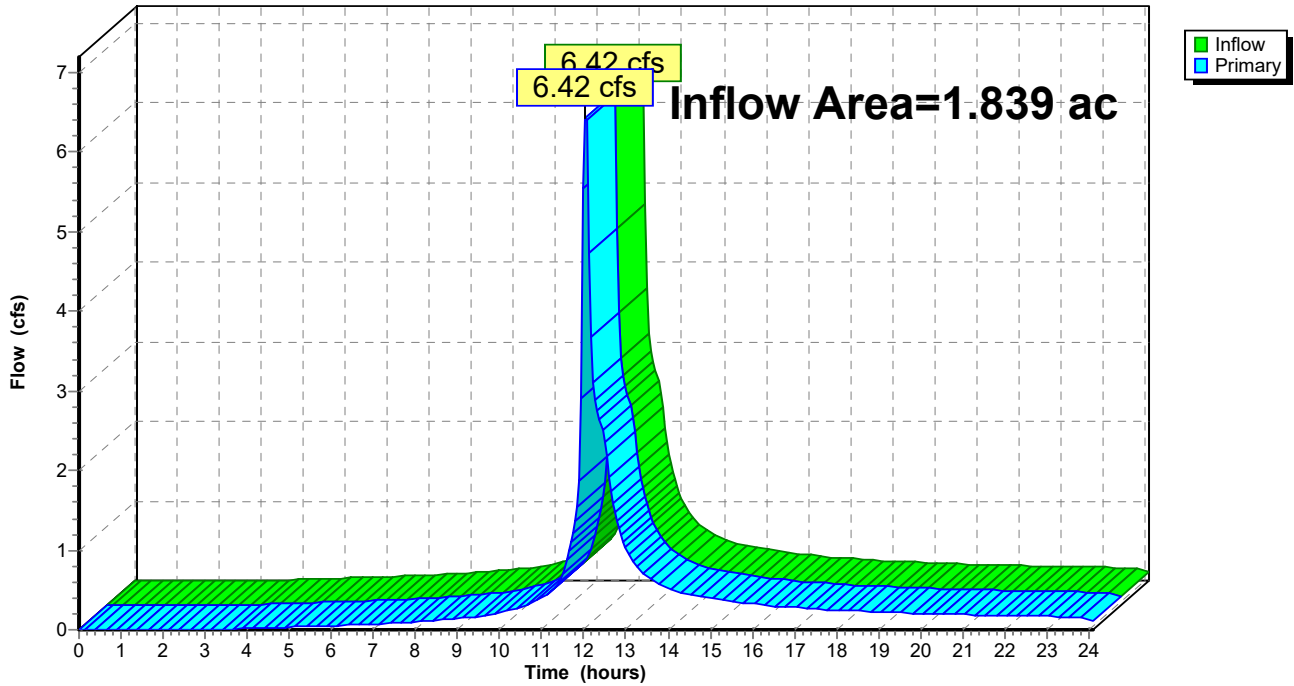
Summary for Link A: EX Site

Inflow Area = 1.839 ac, 18.10% Impervious, Inflow Depth > 4.11" for 50-yr event
Inflow = 6.42 cfs @ 12.04 hrs, Volume= 0.630 af
Primary = 6.42 cfs @ 12.04 hrs, Volume= 0.630 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs

Link A: EX Site

Hydrograph



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 100-yr Rainfall=8.01"

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10: EXWS 10

Runoff Area=39,500 sf 25.32% Impervious Runoff Depth>6.34"
Tc=6.0 min CN=86 Runoff=6.88 cfs 0.479 af

Subcatchment 11: EXWS 11

Runoff Area=40,600 sf 11.08% Impervious Runoff Depth>3.42"
Tc=32.1 min CN=61 Runoff=1.80 cfs 0.266 af

Link A: EX Site

Inflow=7.41 cfs 0.745 af
Primary=7.41 cfs 0.745 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.745 af Average Runoff Depth = 4.86"
81.90% Pervious = 1.506 ac 18.10% Impervious = 0.333 ac

49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 100-yr Rainfall=8.01"

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Summary for Subcatchment 10: EXWS 10

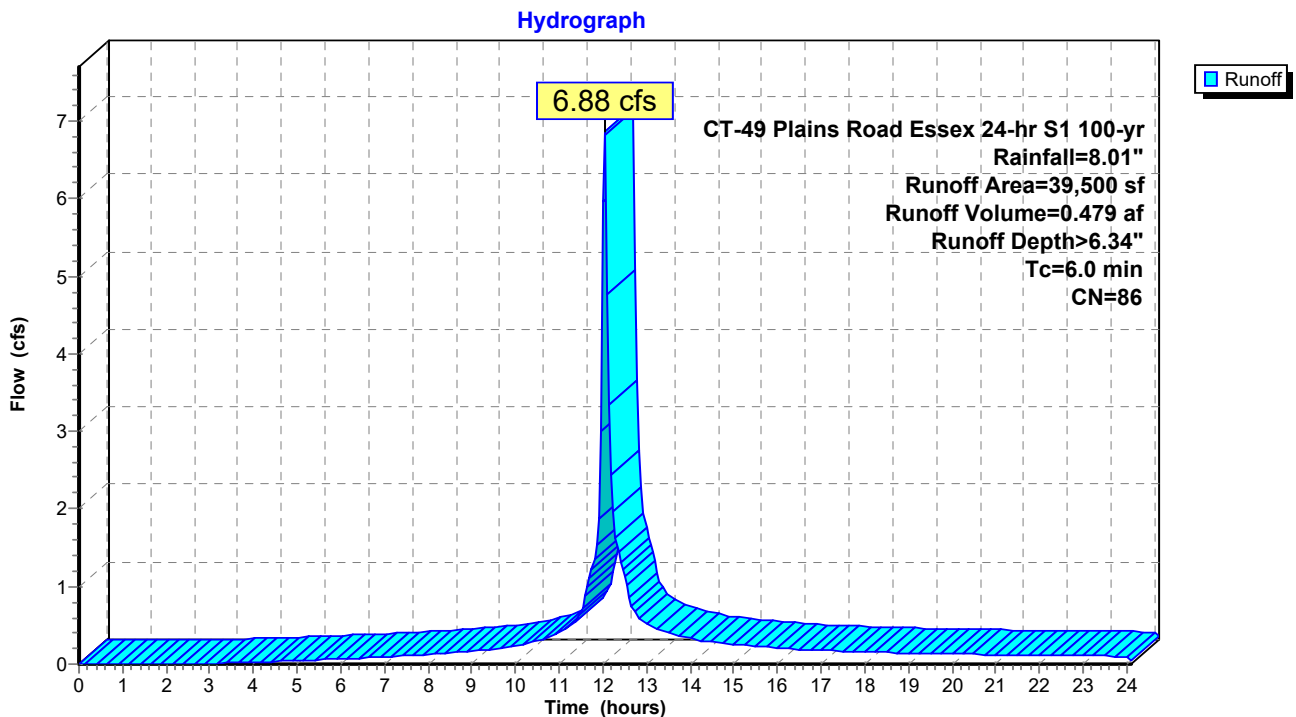
Runoff = 6.88 cfs @ 12.04 hrs, Volume= 0.479 af, Depth> 6.34"
Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
CT-49 Plains Road Essex 24-hr S1 100-yr Rainfall=8.01"

Area (sf)	CN	Description
9,000	55	Woods, Good, HSG B
1,200	61	>75% Grass cover, Good, HSG B
19,300	96	Gravel surface, HSG B
* 10,000	98	Impervious
39,500	86	Weighted Average
29,500		74.68% Pervious Area
10,000		25.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, MIN TR-55 TC 6.0 MIN

Subcatchment 10: EXWS 10



49 Plains Road Existing

CT-49 Plains Road Essex 24-hr S1 100-yr Rainfall=8.01"

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Summary for Subcatchment 11: EXWS 11

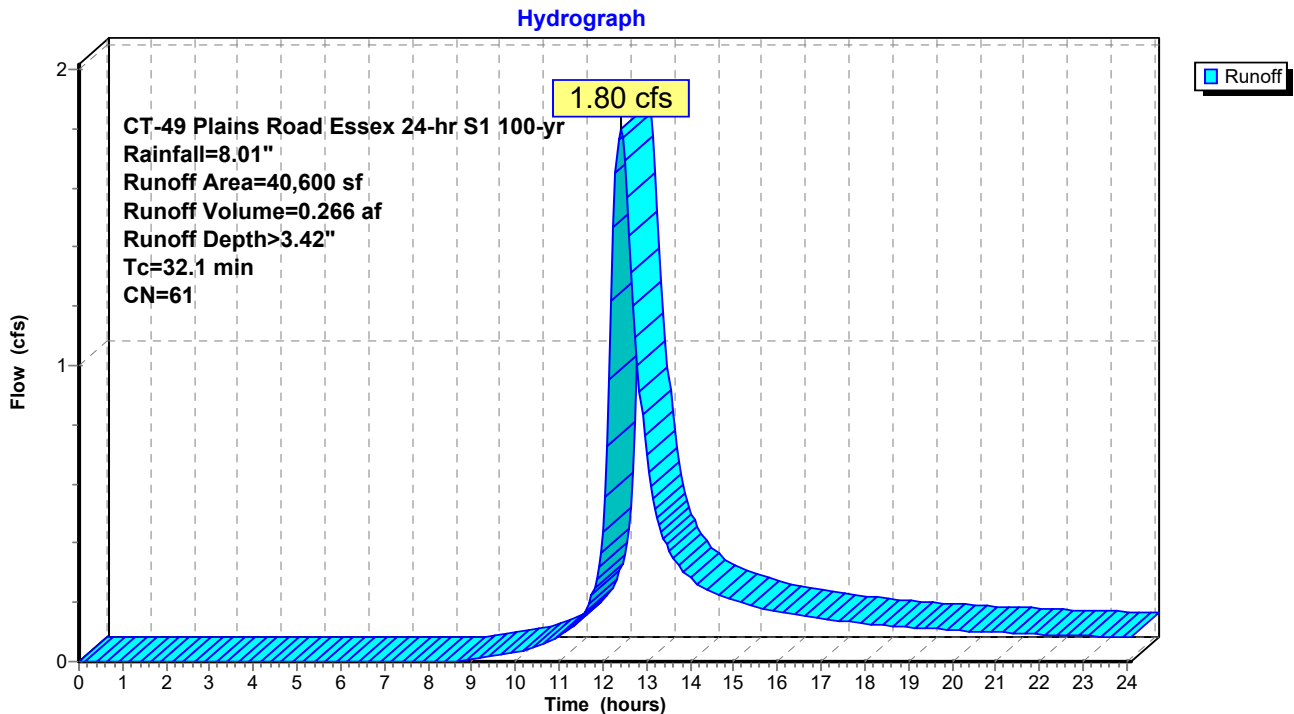
Runoff = 1.80 cfs @ 12.40 hrs, Volume= 0.266 af, Depth> 3.42"
 Routed to Link A : EX Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 100-yr Rainfall=8.01"

Area (sf)	CN	Description
30,500	55	Woods, Good, HSG B
5,600	61	>75% Grass cover, Good, HSG B
* 4,500	98	Impervious
40,600	61	Weighted Average
36,100		88.92% Pervious Area
4,500		11.08% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.1					Direct Entry, See Worksheet

Subcatchment 11: EXWS 11



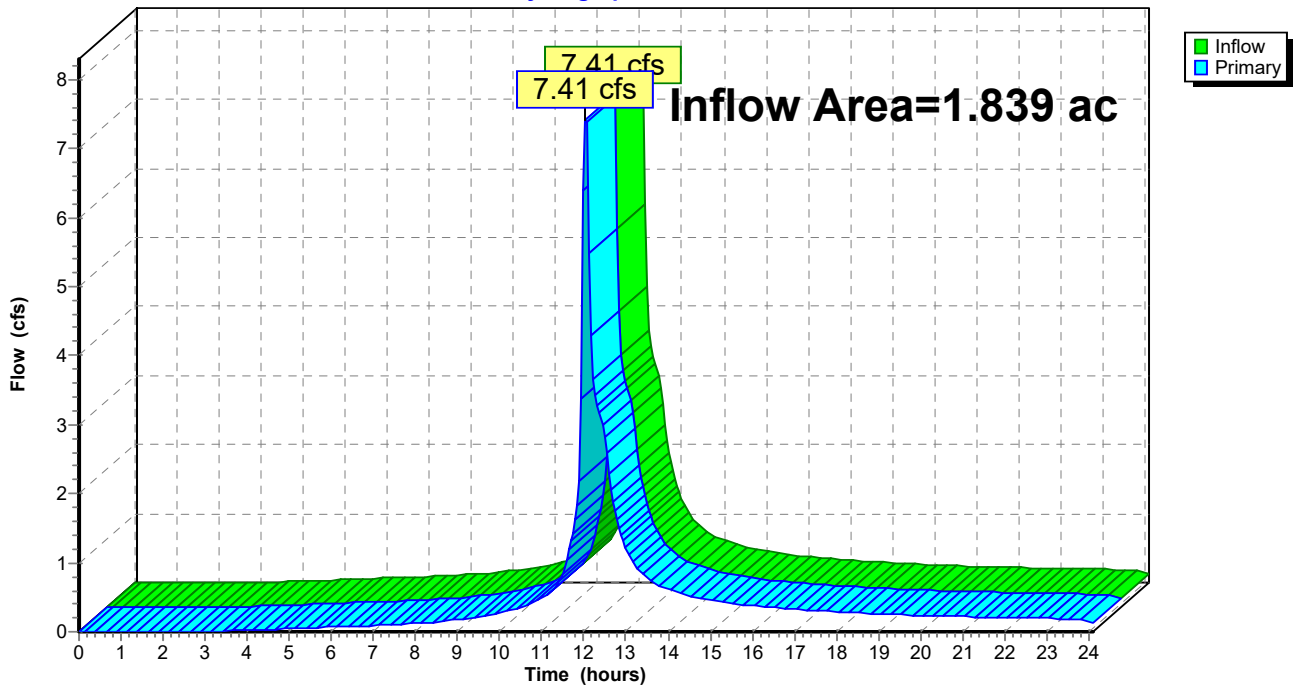
Summary for Link A: EX Site

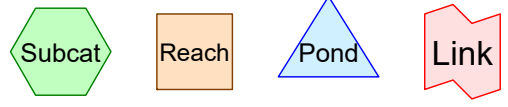
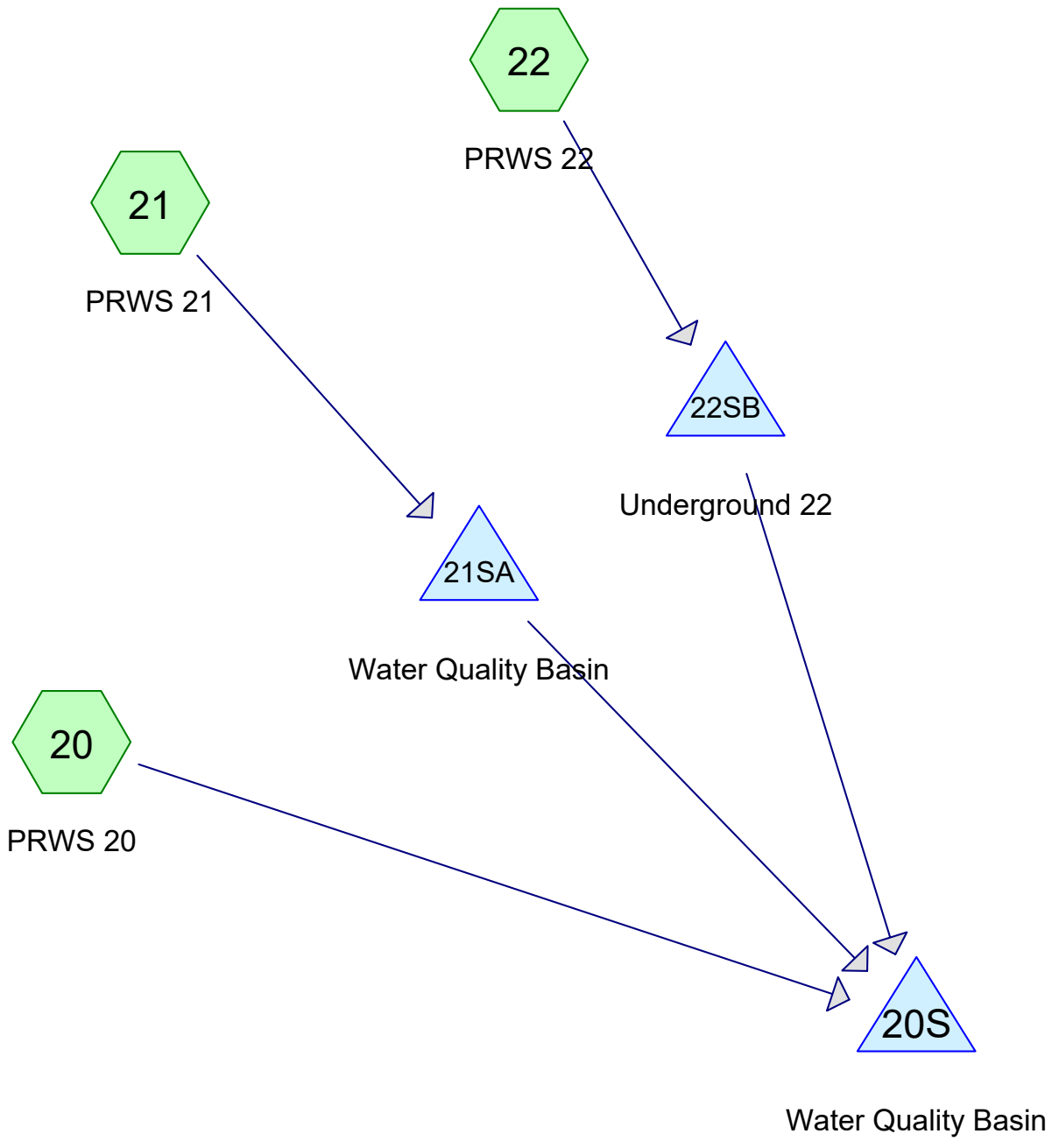
Inflow Area = 1.839 ac, 18.10% Impervious, Inflow Depth > 4.86" for 100-yr event
Inflow = 7.41 cfs @ 12.04 hrs, Volume= 0.745 af
Primary = 7.41 cfs @ 12.04 hrs, Volume= 0.745 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs

Link A: EX Site

Hydrograph





Routing Diagram for 49 Plains Road Proposed Infiltration
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49 Plains Road Proposed Infiltration

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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-yr	CT-49 Plains Road Essex 24-hr S1	1-yr	Default	24.00	1	2.85	2
2	2-yr	CT-49 Plains Road Essex 24-hr S1	2-yr	Default	24.00	1	3.44	2
3	5-yr	CT-49 Plains Road Essex 24-hr S1	5-yr	Default	24.00	1	4.40	2
4	10-yr	CT-49 Plains Road Essex 24-hr S1	10-yr	Default	24.00	1	5.20	2
5	25-yr	CT-49 Plains Road Essex 24-hr S1	25-yr	Default	24.00	1	6.31	2
6	50-yr	CT-49 Plains Road Essex 24-hr S1	50-yr	Default	24.00	1	7.13	2
7	100-yr	CT-49 Plains Road Essex 24-hr S1	100-yr	Default	24.00	1	8.01	2

49 Plains Road Proposed Infiltration

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.560	61	>75% Grass cover, Good, HSG B (20, 21, 22)
0.937	98	Paved parking, HSG B (20, 21, 22)
0.287	98	Roofs, HSG B (22)
0.055	98	Unconnected roofs, HSG B (20)
1.839	87	TOTAL AREA

49 Plains Road Proposed Infiltration

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.560	0.000	0.000	0.000	0.560	>75% Grass cover, Good	20, 21, 22
0.000	0.937	0.000	0.000	0.000	0.937	Paved parking	20, 21, 22
0.000	0.287	0.000	0.000	0.000	0.287	Roofs	22
0.000	0.055	0.000	0.000	0.000	0.055	Unconnected roofs	20
0.000	1.839	0.000	0.000	0.000	1.839	TOTAL AREA	

49 Plains Road Proposed Infiltration *CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"*

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment20: PRWS 20	Runoff Area=46,530 sf 68.29% Impervious Runoff Depth>1.53" Tc=6.0 min CN=86 Runoff=2.07 cfs 0.137 af
Subcatchment21: PRWS 21	Runoff Area=17,460 sf 64.15% Impervious Runoff Depth>1.46" Tc=0.0 min CN=85 Runoff=0.89 cfs 0.049 af
Subcatchment22: PRWS 22	Runoff Area=16,110 sf 78.90% Impervious Runoff Depth>1.85" Tc=6.0 min CN=90 Runoff=0.86 cfs 0.057 af
Pond 20S: Water Quality Basin	Peak Elev=33.64' Storage=6,467 cf Inflow=2.57 cfs 0.185 af Outflow=0.46 cfs 0.128 af
Pond 21SA: Water Quality Basin	Peak Elev=37.42' Storage=1,929 cf Inflow=0.89 cfs 0.049 af Outflow=0.85 cfs 0.049 af
Pond 22SB: Underground 22	Peak Elev=37.61' Storage=0.017 af Inflow=0.86 cfs 0.057 af Discarded=0.09 cfs 0.057 af Primary=0.00 cfs 0.000 af Outflow=0.09 cfs 0.057 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.242 af Average Runoff Depth = 1.58"
30.48% Pervious = 0.560 ac 69.52% Impervious = 1.278 ac

Summary for Subcatchment 20: PRWS 20

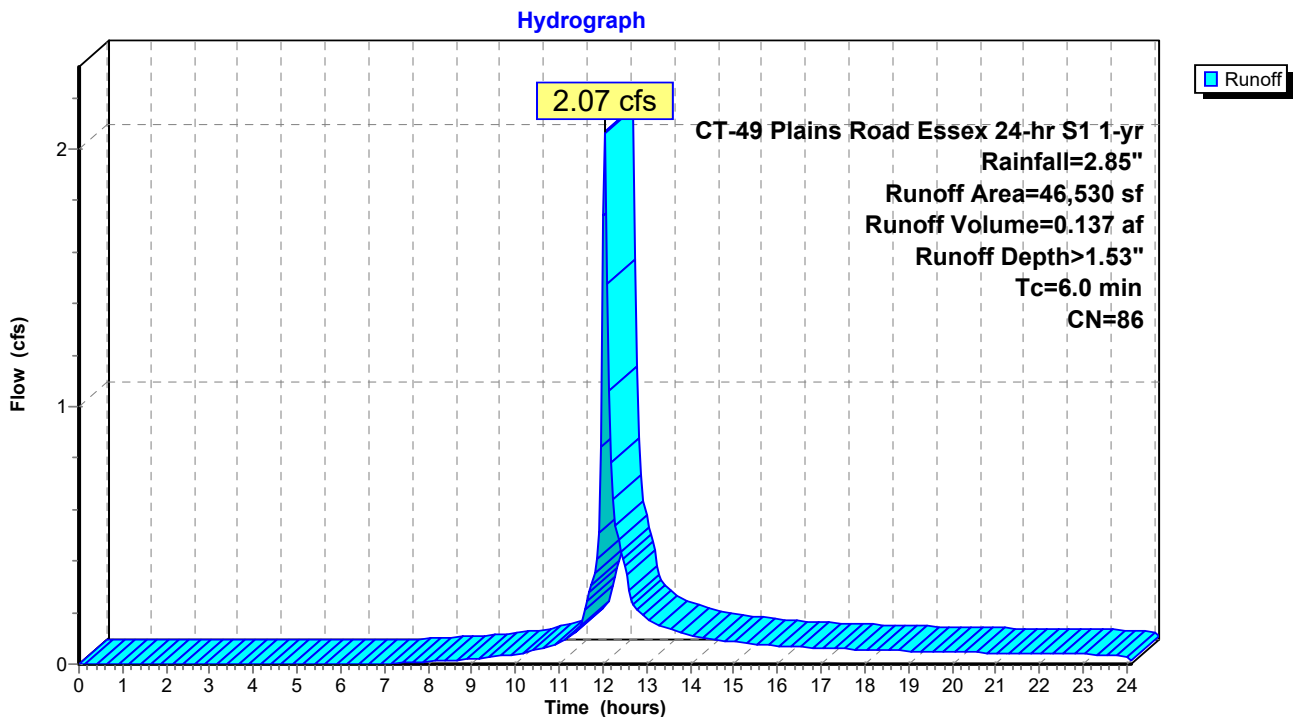
Runoff = 2.07 cfs @ 12.04 hrs, Volume= 0.137 af, Depth> 1.53"
 Routed to Pond 20S : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"

Area (sf)	CN	Description
14,755	61	>75% Grass cover, Good, HSG B
29,400	98	Paved parking, HSG B
2,375	98	Unconnected roofs, HSG B
46,530	86	Weighted Average
14,755		31.71% Pervious Area
31,775		68.29% Impervious Area
2,375		7.47% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment 20: PRWS 20



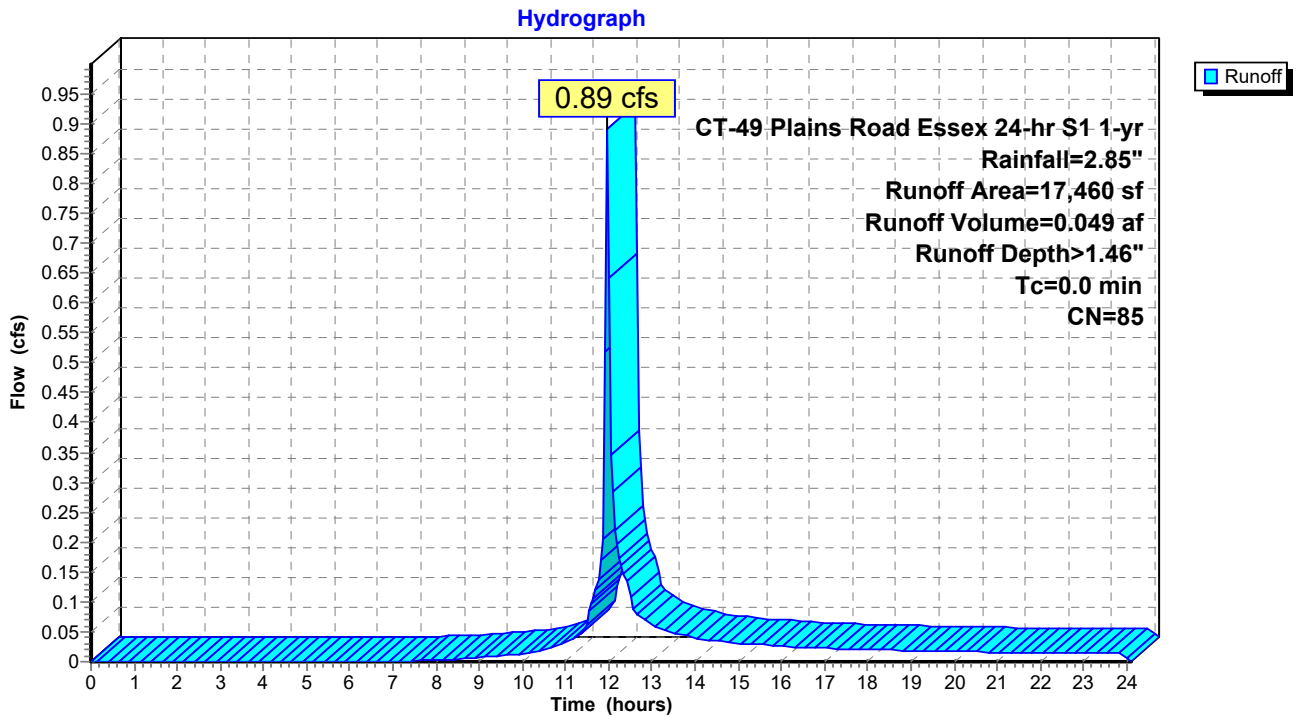
Summary for Subcatchment 21: PRWS 21

Runoff = 0.89 cfs @ 11.96 hrs, Volume= 0.049 af, Depth> 1.46"
 Routed to Pond 21SA : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"

Area (sf)	CN	Description
6,260	61	>75% Grass cover, Good, HSG B
11,200	98	Paved parking, HSG B
17,460	85	Weighted Average
6,260		35.85% Pervious Area
11,200		64.15% Impervious Area

Subcatchment 21: PRWS 21



Summary for Subcatchment 22: PRWS 22

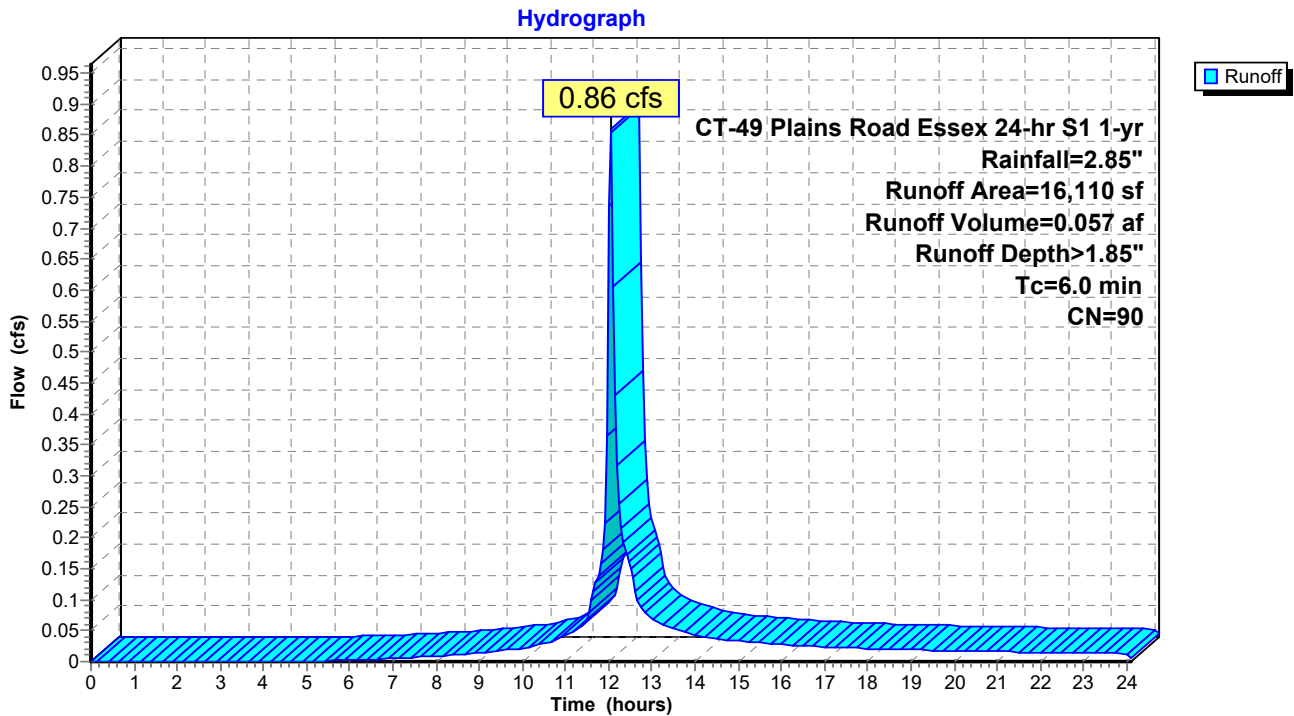
Runoff = 0.86 cfs @ 12.04 hrs, Volume= 0.057 af, Depth> 1.85"
 Routed to Pond 22SB : Underground 22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 1-yr Rainfall=2.85"

Area (sf)	CN	Description
12,500	98	Roofs, HSG B
210	98	Paved parking, HSG B
3,400	61	>75% Grass cover, Good, HSG B
16,110	90	Weighted Average
3,400		21.10% Pervious Area
12,710		78.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TR-55 TC

Subcatchment 22: PRWS 22



Summary for Pond 20S: Water Quality Basin

Inflow Area = 1.839 ac, 69.52% Impervious, Inflow Depth > 1.21" for 1-yr event
 Inflow = 2.57 cfs @ 12.02 hrs, Volume= 0.185 af
 Outflow = 0.46 cfs @ 12.53 hrs, Volume= 0.128 af, Atten= 82%, Lag= 30.6 min
 Primary = 0.46 cfs @ 12.53 hrs, Volume= 0.128 af
 Routed to nonexistent node 30

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 32.80' Surf.Area= 3,942 sf Storage= 2,955 cf
 Peak Elev= 33.64' @ 12.53 hrs Surf.Area= 4,465 sf Storage= 6,467 cf (3,512 cf above start)

Plug-Flow detention time= 444.4 min calculated for 0.060 af (32% of inflow)
 Center-of-Mass det. time= 105.9 min (954.5 - 848.6)

Volume	Invert	Avail.Storage	Storage Description			
#1	32.00'	13,158 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
32.00	3,450	299.0	0	0	3,450	
33.00	4,070	311.0	3,756	3,756	4,108	
34.00	4,700	322.0	4,381	8,137	4,746	
35.00	5,350	335.0	5,021	13,158	5,501	

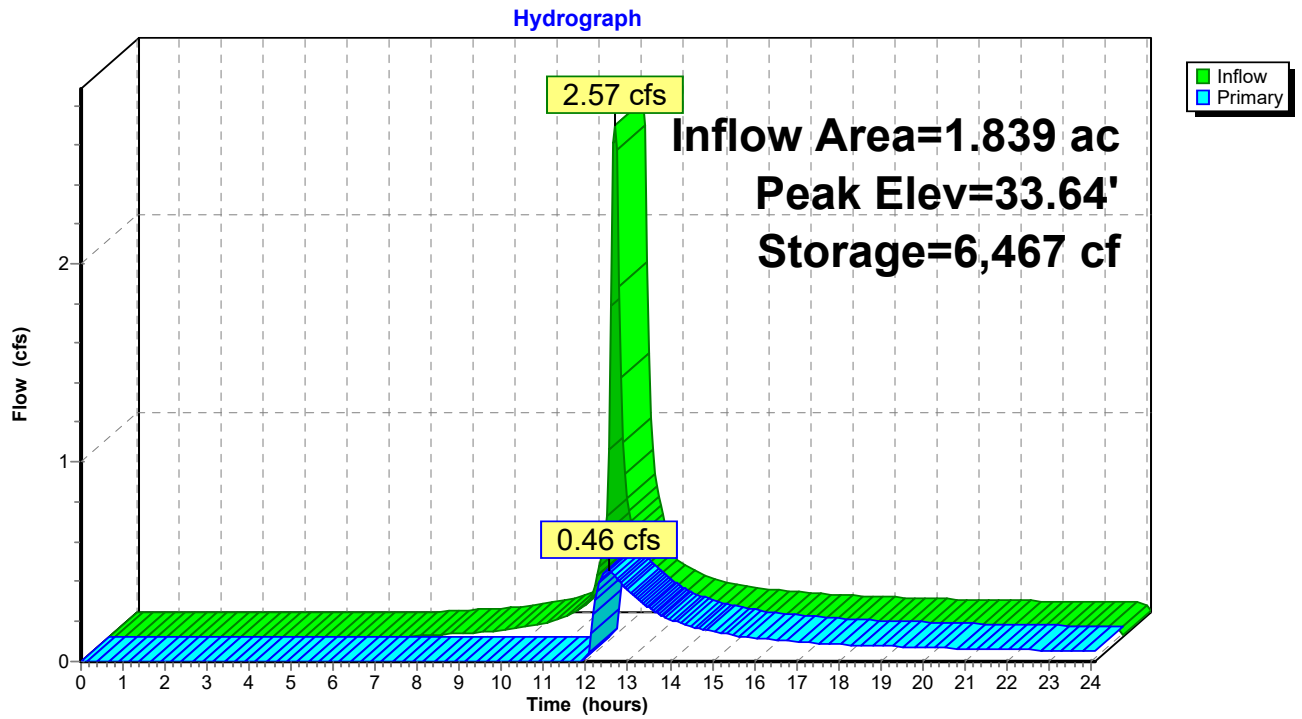
Device	Routing	Invert	Outlet Devices											
#1	Primary	34.50'	10.0' long + 0.5 ' SideZ x 3.0' breadth Broad-Crested Rectangular Weir											
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00											
			2.50 3.00 3.50 4.00 4.50											
			Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68											
			2.72 2.81 2.92 2.97 3.07 3.32											
#2	Primary	33.30'	12.0" Vert. Orifice/Grate C= 0.600											
			Limited to weir flow at low heads											

Primary OutFlow Max=0.46 cfs @ 12.53 hrs HW=33.64' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Orifice/Grate (Orifice Controls 0.46 cfs @ 1.97 fps)

Pond 20S: Water Quality Basin



Summary for Pond 21SA: Water Quality Basin

Inflow Area = 0.401 ac, 64.15% Impervious, Inflow Depth > 1.46" for 1-yr event
 Inflow = 0.89 cfs @ 11.96 hrs, Volume= 0.049 af
 Outflow = 0.85 cfs @ 11.97 hrs, Volume= 0.049 af, Atten= 5%, Lag= 0.7 min
 Primary = 0.85 cfs @ 11.97 hrs, Volume= 0.049 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 37.40' Surf.Area= 1,493 sf Storage= 1,899 cf
 Peak Elev= 37.42' @ 11.97 hrs Surf.Area= 1,504 sf Storage= 1,929 cf (29 cf above start)

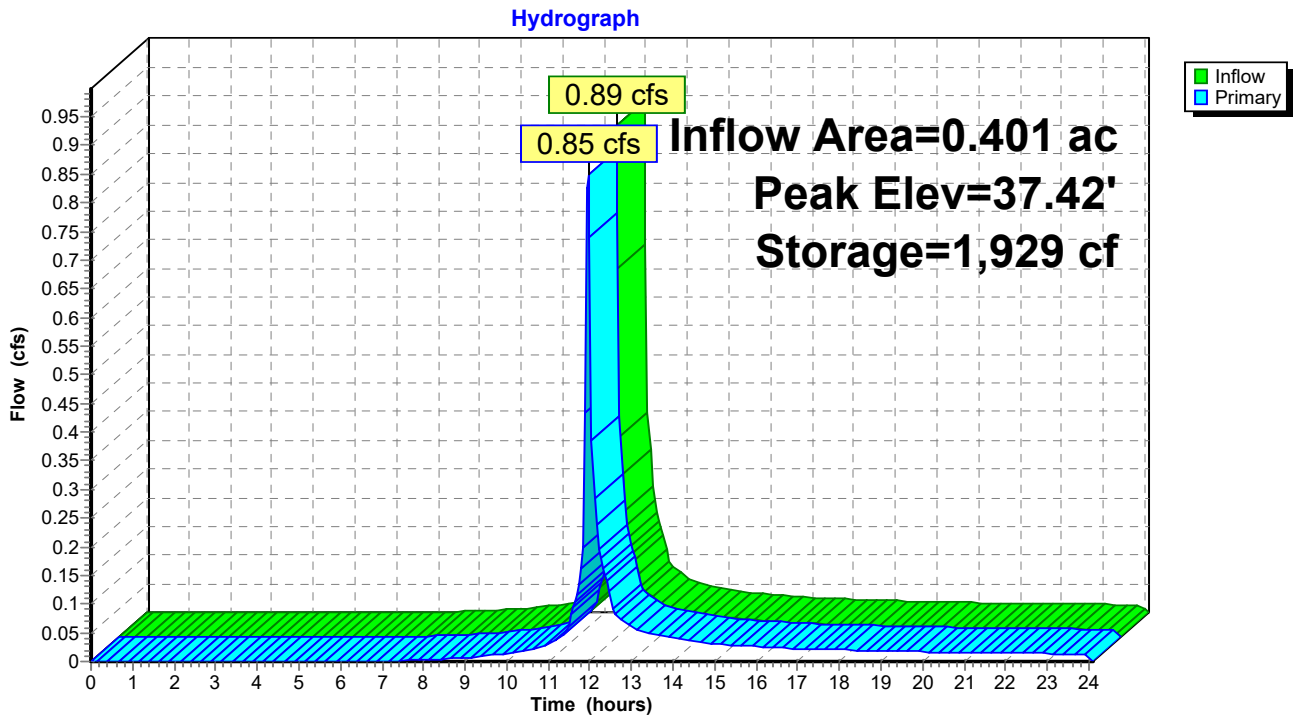
Plug-Flow detention time= 675.4 min calculated for 0.005 af (11% of inflow)
 Center-of-Mass det. time= 0.6 min (848.5 - 847.9)

Volume	Invert	Avail.Storage	Storage Description			
#1	35.00'	2,903 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
35.00	166	238.0	0	0	166	
36.00	681	264.0	394	394	1,235	
37.00	1,259	291.0	955	1,350	2,459	
37.50	1,554	298.0	702	2,052	2,819	
38.00	1,856	304.0	851	2,903	3,143	

Device	Routing	Invert	Outlet Devices
#1	Primary	37.40'	2.4" x 4.0" Horiz. Orifice/Grate X 8.00 columns X 9 rows C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.62 cfs @ 11.97 hrs HW=37.42' (Free Discharge)
 ↑1=Orifice/Grate (Weir Controls 0.62 cfs @ 0.44 fps)

Pond 21SA: Water Quality Basin



Summary for Pond 22SB: Underground 22

Inflow Area = 0.370 ac, 78.90% Impervious, Inflow Depth > 1.85" for 1-yr event
 Inflow = 0.86 cfs @ 12.04 hrs, Volume= 0.057 af
 Outflow = 0.09 cfs @ 12.69 hrs, Volume= 0.057 af, Atten= 89%, Lag= 39.1 min
 Discarded = 0.09 cfs @ 12.69 hrs, Volume= 0.057 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Peak Elev= 37.61' @ 12.69 hrs Surf.Area= 0.072 ac Storage= 0.017 af

Plug-Flow detention time= 56.5 min calculated for 0.057 af (100% of inflow)
 Center-of-Mass det. time= 55.9 min (883.8 - 827.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	37.00'	0.034 af	27.00'W x 116.39'L x 2.00'H Field A 0.144 af Overall - 0.030 af Embedded = 0.114 af x 30.0% Voids
#2A	37.50'	0.030 af	ADS_StormTech SC-160LP +Cap x 192 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 192 Chambers in 12 Rows
		0.064 af	Total Available Storage

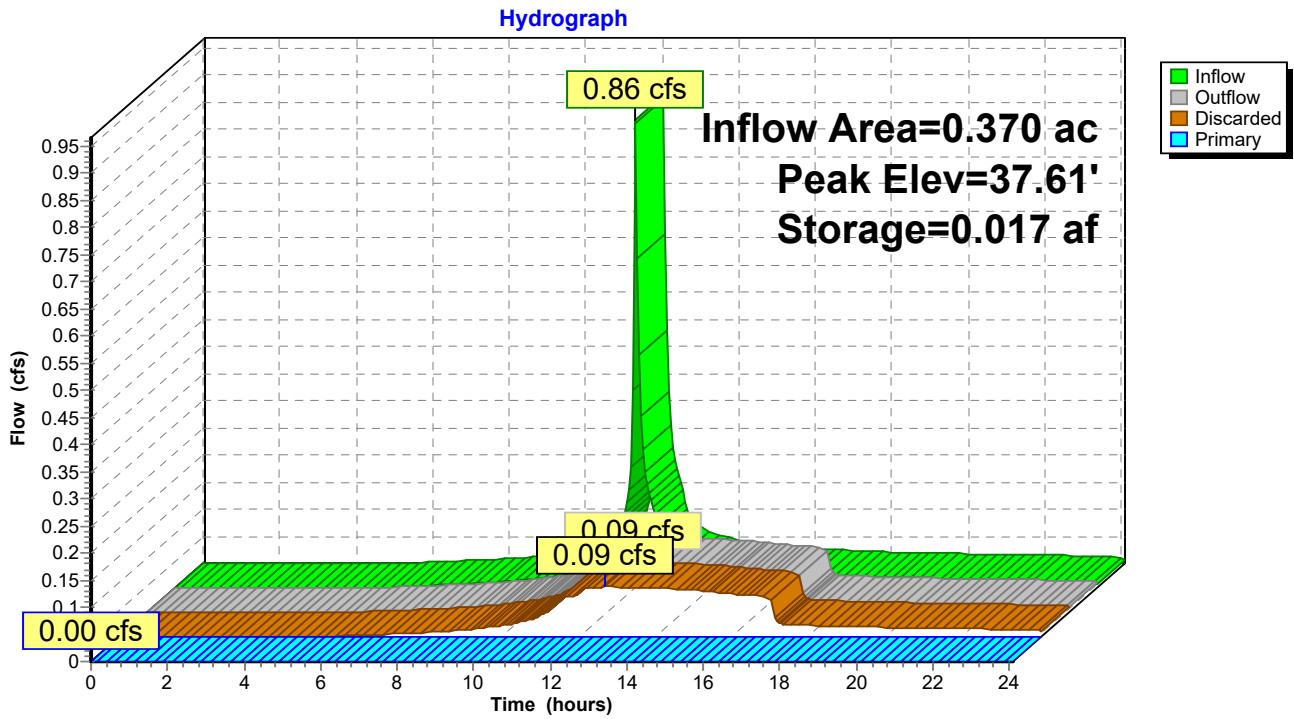
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	37.00'	1.000 in/hr Exfiltration over Wetted area Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.40'	4.0' long + 1.0 ' SideZ x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.09 cfs @ 12.69 hrs HW=37.61' (Free Discharge)
 ↑1=Exfiltration (Controls 0.09 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=37.00' (Free Discharge)
 ↑2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 22SB: Underground 22



49 Plains Road Proposed Infiltration *CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"*

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment20: PRWS 20	Runoff Area=46,530 sf 68.29% Impervious Runoff Depth>2.05" Tc=6.0 min CN=86 Runoff=2.75 cfs 0.182 af
Subcatchment21: PRWS 21	Runoff Area=17,460 sf 64.15% Impervious Runoff Depth>1.96" Tc=0.0 min CN=85 Runoff=1.20 cfs 0.066 af
Subcatchment22: PRWS 22	Runoff Area=16,110 sf 78.90% Impervious Runoff Depth>2.39" Tc=6.0 min CN=90 Runoff=1.10 cfs 0.074 af
Pond 20S: Water Quality Basin	Peak Elev=33.80' Storage=7,225 cf Inflow=3.42 cfs 0.248 af Outflow=0.96 cfs 0.189 af
Pond 21SA: Water Quality Basin	Peak Elev=37.43' Storage=1,939 cf Inflow=1.20 cfs 0.066 af Outflow=1.14 cfs 0.066 af
Pond 22SB: Underground 22	Peak Elev=37.74' Storage=0.024 af Inflow=1.10 cfs 0.074 af Discarded=0.10 cfs 0.074 af Primary=0.00 cfs 0.000 af Outflow=0.10 cfs 0.074 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.321 af Average Runoff Depth = 2.10"
30.48% Pervious = 0.560 ac 69.52% Impervious = 1.278 ac

Summary for Subcatchment 20: PRWS 20

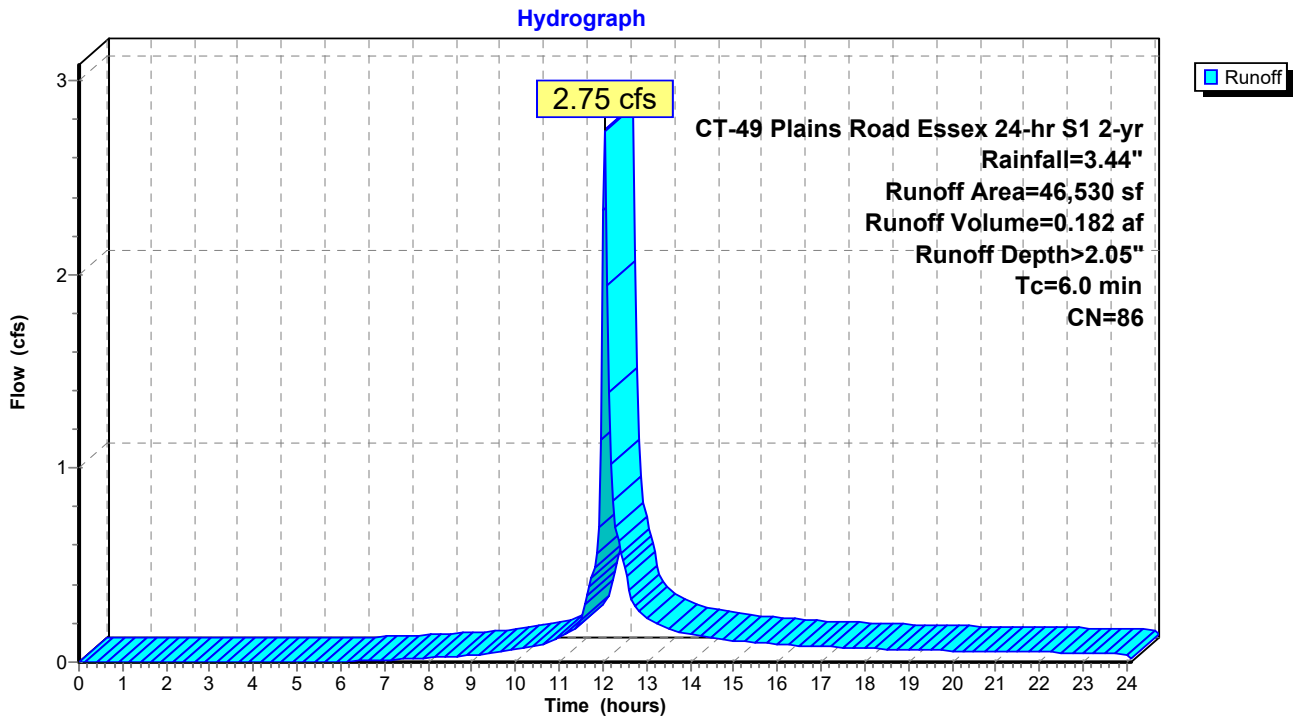
Runoff = 2.75 cfs @ 12.04 hrs, Volume= 0.182 af, Depth> 2.05"
 Routed to Pond 20S : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"

Area (sf)	CN	Description
14,755	61	>75% Grass cover, Good, HSG B
29,400	98	Paved parking, HSG B
2,375	98	Unconnected roofs, HSG B
46,530	86	Weighted Average
14,755		31.71% Pervious Area
31,775		68.29% Impervious Area
2,375		7.47% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment 20: PRWS 20



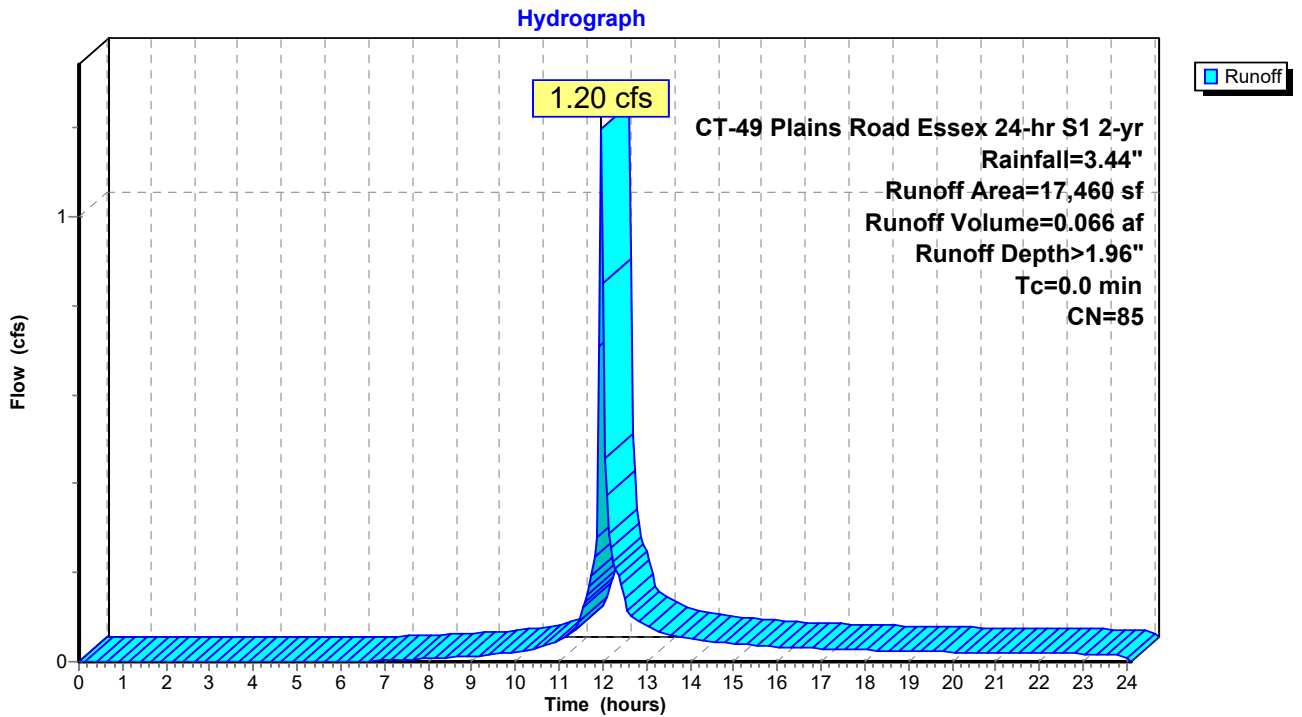
Summary for Subcatchment 21: PRWS 21

Runoff = 1.20 cfs @ 11.95 hrs, Volume= 0.066 af, Depth> 1.96"
 Routed to Pond 21SA : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"

Area (sf)	CN	Description
6,260	61	>75% Grass cover, Good, HSG B
11,200	98	Paved parking, HSG B
17,460	85	Weighted Average
6,260		35.85% Pervious Area
11,200		64.15% Impervious Area

Subcatchment 21: PRWS 21



Summary for Subcatchment 22: PRWS 22

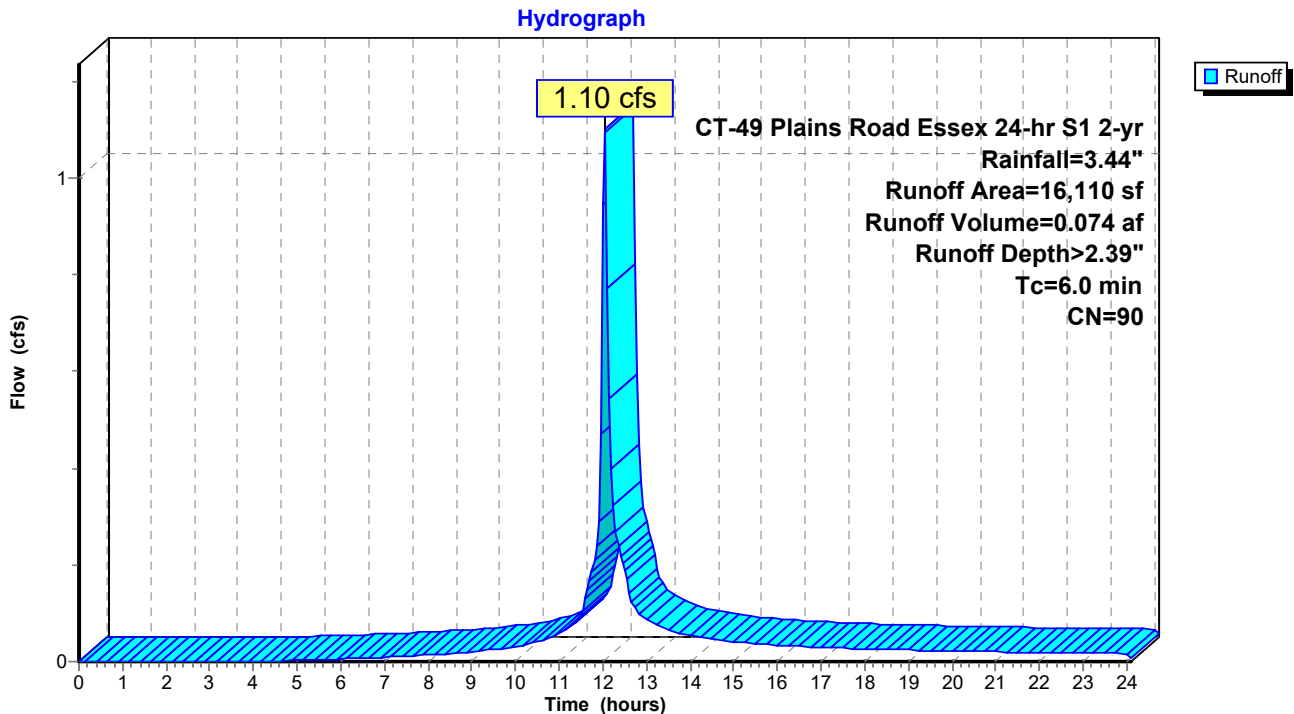
Runoff = 1.10 cfs @ 12.04 hrs, Volume= 0.074 af, Depth> 2.39"
 Routed to Pond 22SB : Underground 22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 2-yr Rainfall=3.44"

Area (sf)	CN	Description
12,500	98	Roofs, HSG B
210	98	Paved parking, HSG B
3,400	61	>75% Grass cover, Good, HSG B
16,110	90	Weighted Average
3,400		21.10% Pervious Area
12,710		78.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TR-55 TC

Subcatchment 22: PRWS 22



Summary for Pond 20S: Water Quality Basin

Inflow Area = 1.839 ac, 69.52% Impervious, Inflow Depth > 1.62" for 2-yr event
 Inflow = 3.42 cfs @ 12.02 hrs, Volume= 0.248 af
 Outflow = 0.96 cfs @ 12.29 hrs, Volume= 0.189 af, Atten= 72%, Lag= 16.0 min
 Primary = 0.96 cfs @ 12.29 hrs, Volume= 0.189 af
 Routed to nonexistent node 30

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 32.80' Surf.Area= 3,942 sf Storage= 2,955 cf
 Peak Elev= 33.80' @ 12.29 hrs Surf.Area= 4,572 sf Storage= 7,225 cf (4,270 cf above start)

Plug-Flow detention time= 327.7 min calculated for 0.121 af (49% of inflow)
 Center-of-Mass det. time= 85.3 min (923.7 - 838.4)

Volume	Invert	Avail.Storage	Storage Description			
#1	32.00'	13,158 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
32.00	3,450	299.0	0	0	3,450	
33.00	4,070	311.0	3,756	3,756	4,108	
34.00	4,700	322.0	4,381	8,137	4,746	
35.00	5,350	335.0	5,021	13,158	5,501	

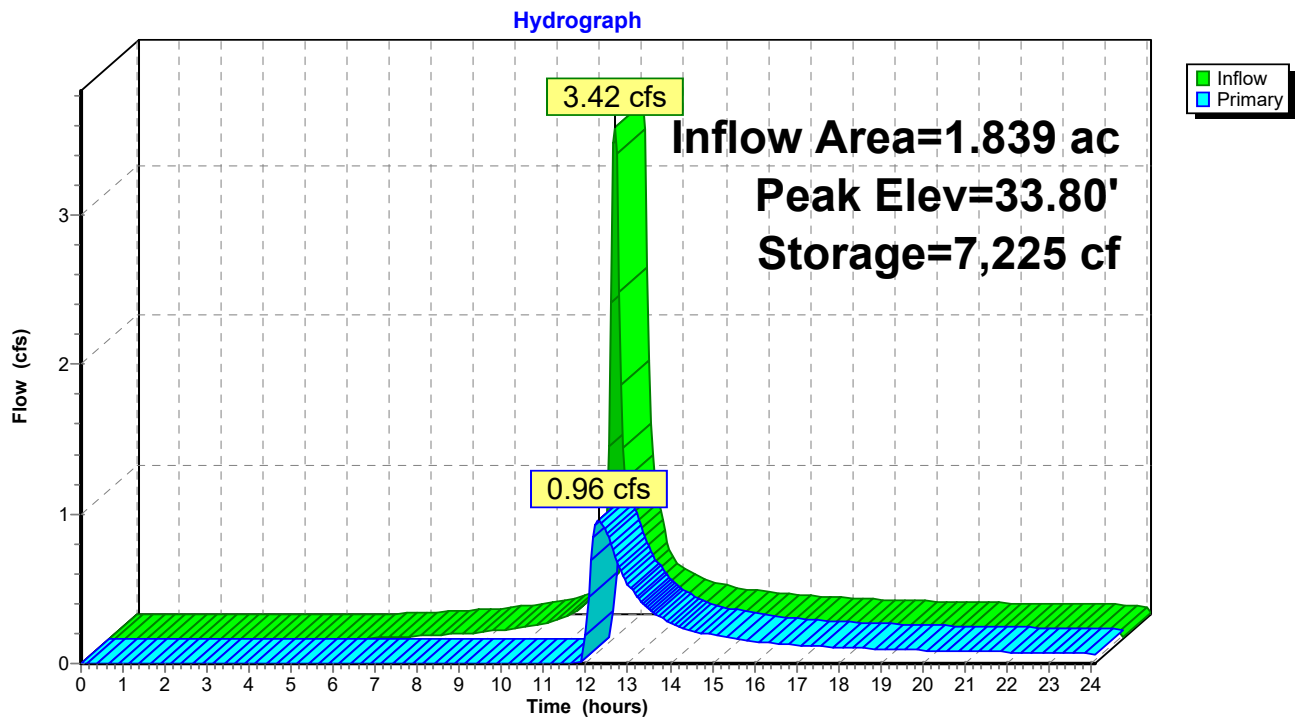
Device	Routing	Invert	Outlet Devices											
#1	Primary	34.50'	10.0' long + 0.5 ' SideZ x 3.0' breadth Broad-Crested Rectangular Weir											
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00											
			2.50 3.00 3.50 4.00 4.50											
			Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68											
			2.72 2.81 2.92 2.97 3.07 3.32											
#2	Primary	33.30'	12.0" Vert. Orifice/Grate C= 0.600											
			Limited to weir flow at low heads											

Primary OutFlow Max=0.96 cfs @ 12.29 hrs HW=33.80' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Orifice/Grate (Orifice Controls 0.96 cfs @ 2.41 fps)

Pond 20S: Water Quality Basin



Summary for Pond 21SA: Water Quality Basin

Inflow Area = 0.401 ac, 64.15% Impervious, Inflow Depth > 1.96" for 2-yr event
 Inflow = 1.20 cfs @ 11.95 hrs, Volume= 0.066 af
 Outflow = 1.14 cfs @ 11.97 hrs, Volume= 0.066 af, Atten= 5%, Lag= 0.7 min
 Primary = 1.14 cfs @ 11.97 hrs, Volume= 0.066 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 37.40' Surf.Area= 1,493 sf Storage= 1,899 cf
 Peak Elev= 37.43' @ 11.97 hrs Surf.Area= 1,508 sf Storage= 1,939 cf (39 cf above start)

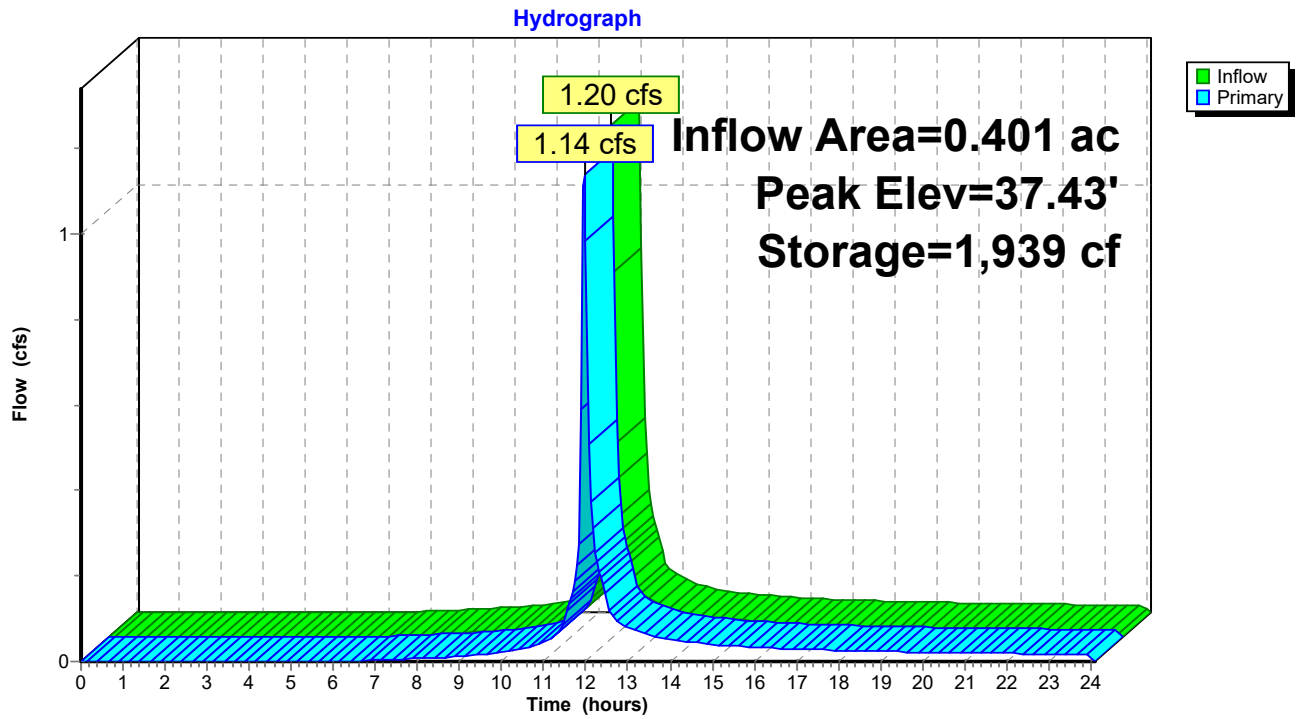
Plug-Flow detention time= 407.8 min calculated for 0.022 af (34% of inflow)
 Center-of-Mass det. time= 0.6 min (838.1 - 837.5)

Volume	Invert	Avail.Storage	Storage Description		
#1	35.00'	2,903 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
35.00	166	238.0	0	0	166
36.00	681	264.0	394	394	1,235
37.00	1,259	291.0	955	1,350	2,459
37.50	1,554	298.0	702	2,052	2,819
38.00	1,856	304.0	851	2,903	3,143

Device	Routing	Invert	Outlet Devices
#1	Primary	37.40'	2.4" x 4.0" Horiz. Orifice/Grate X 8.00 columns X 9 rows C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.97 cfs @ 11.97 hrs HW=37.42' (Free Discharge)
 ↑1=Orifice/Grate (Weir Controls 0.97 cfs @ 0.51 fps)

Pond 21SA: Water Quality Basin



Summary for Pond 22SB: Underground 22

Inflow Area = 0.370 ac, 78.90% Impervious, Inflow Depth > 2.39" for 2-yr event
 Inflow = 1.10 cfs @ 12.04 hrs, Volume= 0.074 af
 Outflow = 0.10 cfs @ 12.87 hrs, Volume= 0.074 af, Atten= 91%, Lag= 49.8 min
 Discarded = 0.10 cfs @ 12.87 hrs, Volume= 0.074 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Peak Elev= 37.74' @ 12.87 hrs Surf.Area= 0.072 ac Storage= 0.024 af

Plug-Flow detention time= 81.9 min calculated for 0.074 af (100% of inflow)
 Center-of-Mass det. time= 81.4 min (900.2 - 818.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	37.00'	0.034 af	27.00'W x 116.39'L x 2.00'H Field A 0.144 af Overall - 0.030 af Embedded = 0.114 af x 30.0% Voids
#2A	37.50'	0.030 af	ADS_StormTech SC-160LP +Cap x 192 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 192 Chambers in 12 Rows
		0.064 af	Total Available Storage

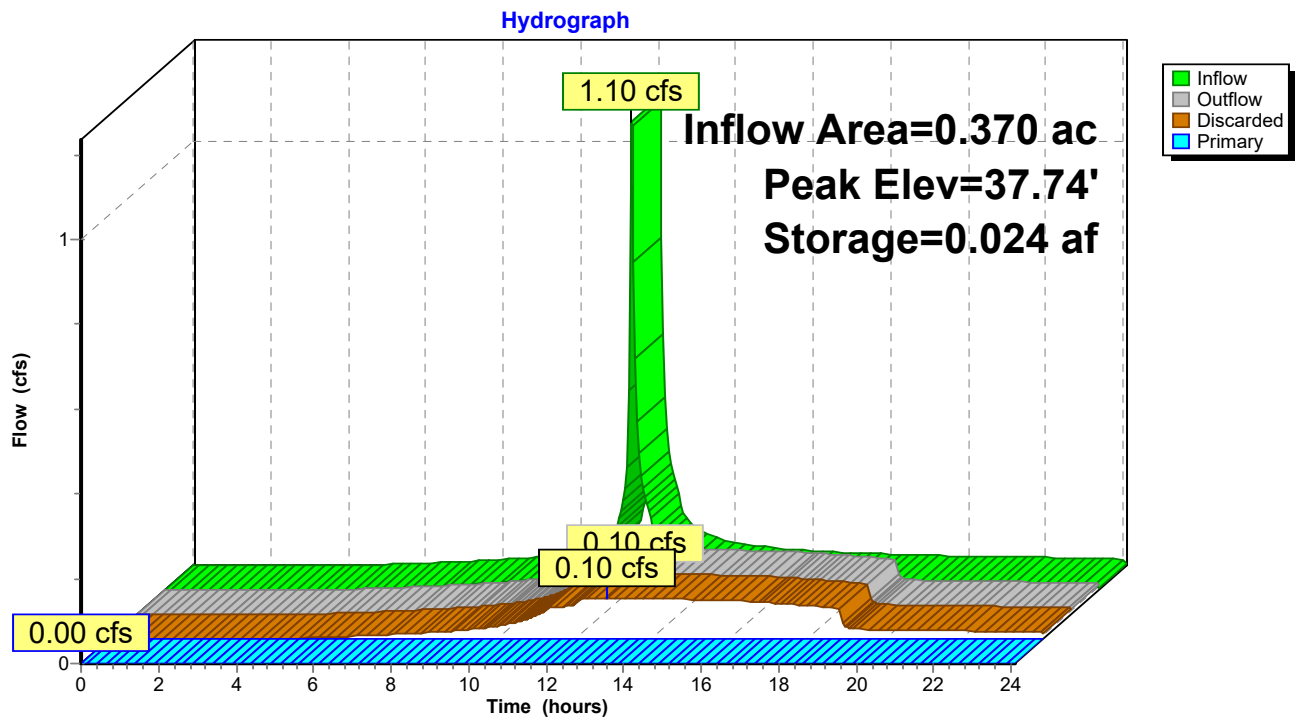
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	37.00'	1.000 in/hr Exfiltration over Wetted area Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.40'	4.0' long + 1.0 ' SideZ x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.10 cfs @ 12.87 hrs HW=37.74' (Free Discharge)
 ↑1=Exfiltration (Controls 0.10 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=37.00' (Free Discharge)
 ↑2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 22SB: Underground 22



49 Plains Road Proposed Infiltration *CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"*

Prepared by Doane Engineering

Printed 2/9/2023

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment20: PRWS 20	Runoff Area=46,530 sf 68.29% Impervious Runoff Depth>2.91" Tc=6.0 min CN=86 Runoff=3.88 cfs 0.259 af
Subcatchment21: PRWS 21	Runoff Area=17,460 sf 64.15% Impervious Runoff Depth>2.82" Tc=0.0 min CN=85 Runoff=1.71 cfs 0.094 af
Subcatchment22: PRWS 22	Runoff Area=16,110 sf 78.90% Impervious Runoff Depth>3.30" Tc=6.0 min CN=90 Runoff=1.50 cfs 0.102 af
Pond 20S: Water Quality Basin	Peak Elev=34.07' Storage=8,479 cf Inflow=4.80 cfs 0.353 af Outflow=1.95 cfs 0.293 af
Pond 21SA: Water Quality Basin	Peak Elev=37.43' Storage=1,952 cf Inflow=1.71 cfs 0.094 af Outflow=1.65 cfs 0.094 af
Pond 22SB: Underground 22	Peak Elev=38.00' Storage=0.036 af Inflow=1.50 cfs 0.102 af Discarded=0.10 cfs 0.102 af Primary=0.00 cfs 0.000 af Outflow=0.10 cfs 0.102 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.455 af Average Runoff Depth = 2.97"
30.48% Pervious = 0.560 ac 69.52% Impervious = 1.278 ac

Summary for Subcatchment 20: PRWS 20

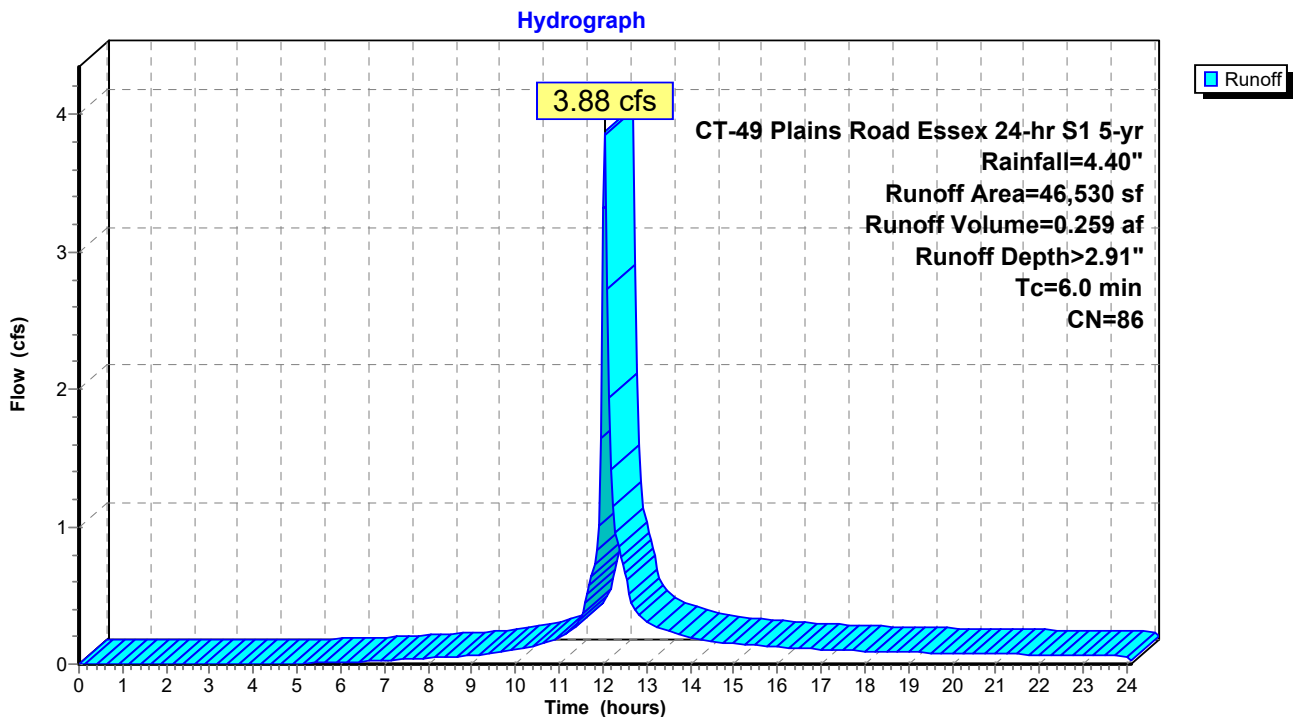
Runoff = 3.88 cfs @ 12.04 hrs, Volume= 0.259 af, Depth> 2.91"
 Routed to Pond 20S : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"

Area (sf)	CN	Description
14,755	61	>75% Grass cover, Good, HSG B
29,400	98	Paved parking, HSG B
2,375	98	Unconnected roofs, HSG B
46,530	86	Weighted Average
14,755		31.71% Pervious Area
31,775		68.29% Impervious Area
2,375		7.47% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment 20: PRWS 20



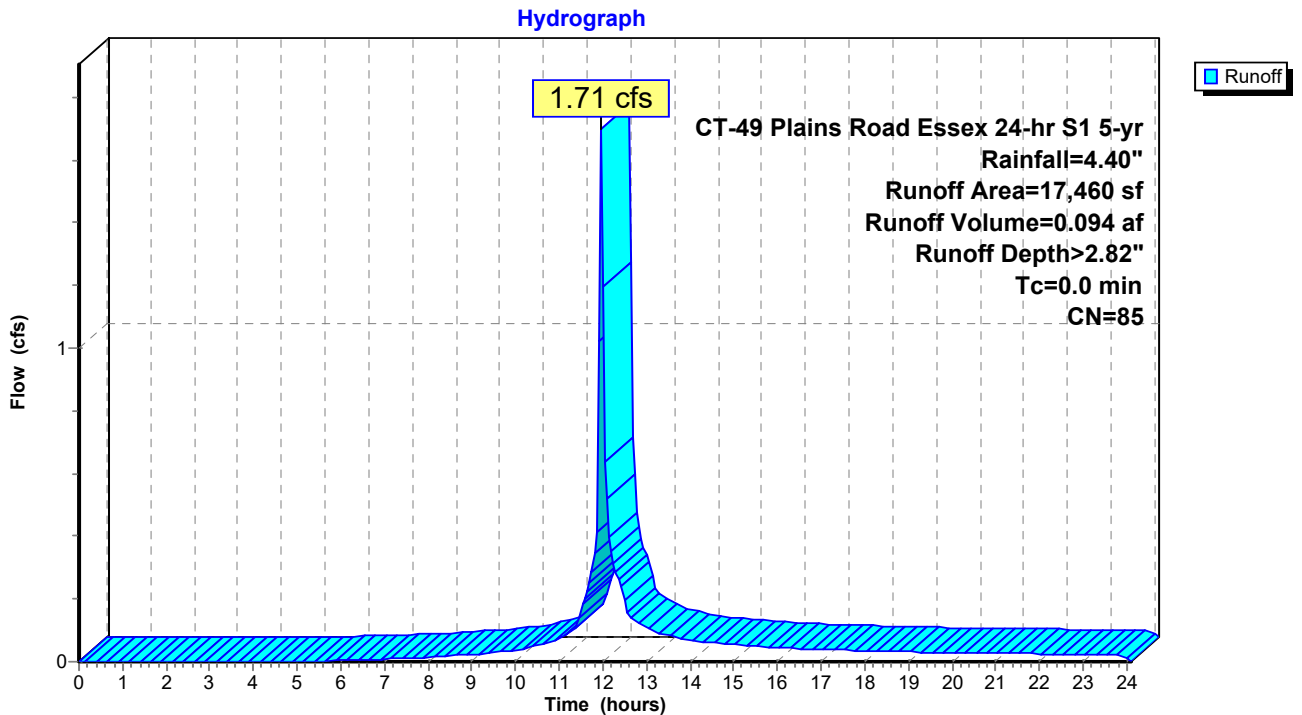
Summary for Subcatchment 21: PRWS 21

Runoff = 1.71 cfs @ 11.95 hrs, Volume= 0.094 af, Depth> 2.82"
 Routed to Pond 21SA : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"

Area (sf)	CN	Description
6,260	61	>75% Grass cover, Good, HSG B
11,200	98	Paved parking, HSG B
17,460	85	Weighted Average
6,260		35.85% Pervious Area
11,200		64.15% Impervious Area

Subcatchment 21: PRWS 21



Summary for Subcatchment 22: PRWS 22

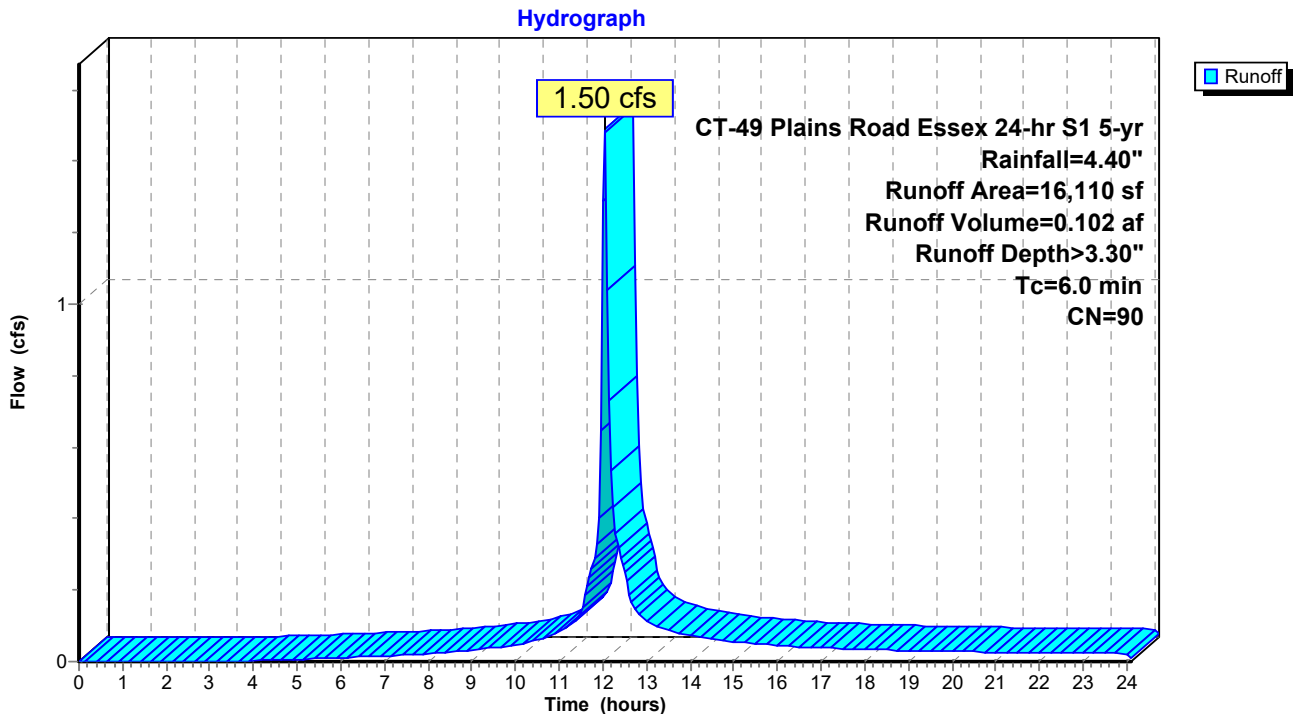
Runoff = 1.50 cfs @ 12.04 hrs, Volume= 0.102 af, Depth> 3.30"
 Routed to Pond 22SB : Underground 22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 5-yr Rainfall=4.40"

Area (sf)	CN	Description
12,500	98	Roofs, HSG B
210	98	Paved parking, HSG B
3,400	61	>75% Grass cover, Good, HSG B
16,110	90	Weighted Average
3,400		21.10% Pervious Area
12,710		78.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TR-55 TC

Subcatchment 22: PRWS 22



Summary for Pond 20S: Water Quality Basin

Inflow Area = 1.839 ac, 69.52% Impervious, Inflow Depth > 2.31" for 5-yr event
 Inflow = 4.80 cfs @ 12.02 hrs, Volume= 0.353 af
 Outflow = 1.95 cfs @ 12.19 hrs, Volume= 0.293 af, Atten= 59%, Lag= 10.0 min
 Primary = 1.95 cfs @ 12.19 hrs, Volume= 0.293 af
 Routed to nonexistent node 30

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 32.80' Surf.Area= 3,942 sf Storage= 2,955 cf
 Peak Elev= 34.07' @ 12.19 hrs Surf.Area= 4,746 sf Storage= 8,479 cf (5,525 cf above start)

Plug-Flow detention time= 246.8 min calculated for 0.224 af (64% of inflow)
 Center-of-Mass det. time= 70.6 min (896.0 - 825.5)

Volume	Invert	Avail.Storage	Storage Description			
#1	32.00'	13,158 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
32.00	3,450	299.0	0	0	3,450	
33.00	4,070	311.0	3,756	3,756	4,108	
34.00	4,700	322.0	4,381	8,137	4,746	
35.00	5,350	335.0	5,021	13,158	5,501	

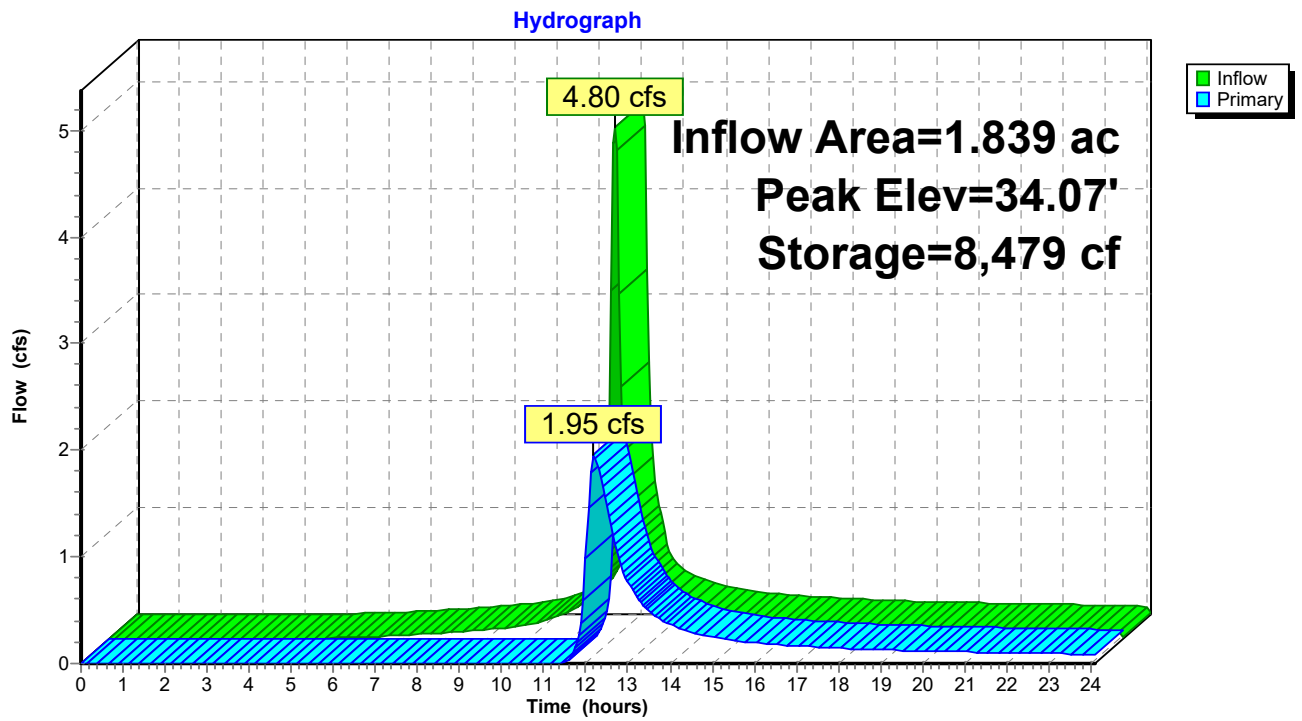
Device	Routing	Invert	Outlet Devices											
#1	Primary	34.50'	10.0' long + 0.5 ' SideZ x 3.0' breadth Broad-Crested Rectangular Weir											
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00											
			2.50 3.00 3.50 4.00 4.50											
			Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68											
			2.72 2.81 2.92 2.97 3.07 3.32											
#2	Primary	33.30'	12.0" Vert. Orifice/Grate C= 0.600											
			Limited to weir flow at low heads											

Primary OutFlow Max=1.94 cfs @ 12.19 hrs HW=34.07' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Orifice/Grate (Orifice Controls 1.94 cfs @ 2.99 fps)

Pond 20S: Water Quality Basin



Summary for Pond 21SA: Water Quality Basin

Inflow Area = 0.401 ac, 64.15% Impervious, Inflow Depth > 2.82" for 5-yr event
 Inflow = 1.71 cfs @ 11.95 hrs, Volume= 0.094 af
 Outflow = 1.65 cfs @ 11.96 hrs, Volume= 0.094 af, Atten= 3%, Lag= 0.5 min
 Primary = 1.65 cfs @ 11.96 hrs, Volume= 0.094 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 37.40' Surf.Area= 1,493 sf Storage= 1,899 cf
 Peak Elev= 37.43' @ 11.96 hrs Surf.Area= 1,514 sf Storage= 1,952 cf (52 cf above start)

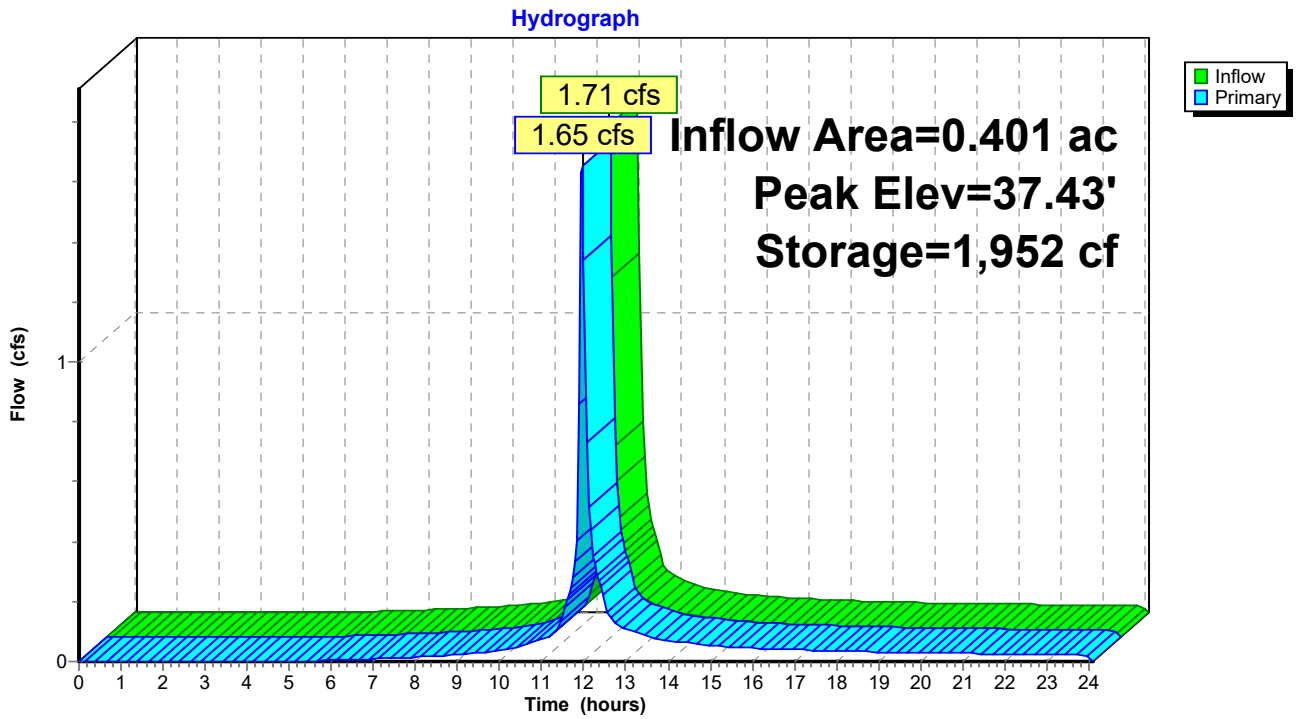
Plug-Flow detention time= 267.1 min calculated for 0.050 af (54% of inflow)
 Center-of-Mass det. time= 0.6 min (824.9 - 824.4)

Volume	Invert	Avail.Storage	Storage Description			
#1	35.00'	2,903 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
35.00	166	238.0	0	0	166	
36.00	681	264.0	394	394	1,235	
37.00	1,259	291.0	955	1,350	2,459	
37.50	1,554	298.0	702	2,052	2,819	
38.00	1,856	304.0	851	2,903	3,143	

Device	Routing	Invert	Outlet Devices
#1	Primary	37.40'	2.4" x 4.0" Horiz. Orifice/Grate X 8.00 columns X 9 rows C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.52 cfs @ 11.96 hrs HW=37.43' (Free Discharge)
 ↑1=Orifice/Grate (Weir Controls 1.52 cfs @ 0.60 fps)

Pond 21SA: Water Quality Basin



Summary for Pond 22SB: Underground 22

Inflow Area = 0.370 ac, 78.90% Impervious, Inflow Depth > 3.30" for 5-yr event
 Inflow = 1.50 cfs @ 12.04 hrs, Volume= 0.102 af
 Outflow = 0.10 cfs @ 13.16 hrs, Volume= 0.102 af, Atten= 93%, Lag= 66.9 min
 Discarded = 0.10 cfs @ 13.16 hrs, Volume= 0.102 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Peak Elev= 38.00' @ 13.16 hrs Surf.Area= 0.072 ac Storage= 0.036 af

Plug-Flow detention time= 128.2 min calculated for 0.101 af (100% of inflow)
 Center-of-Mass det. time= 127.5 min (934.8 - 807.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	37.00'	0.034 af	27.00'W x 116.39'L x 2.00'H Field A 0.144 af Overall - 0.030 af Embedded = 0.114 af x 30.0% Voids
#2A	37.50'	0.030 af	ADS_StormTech SC-160LP +Cap x 192 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 192 Chambers in 12 Rows
		0.064 af	Total Available Storage

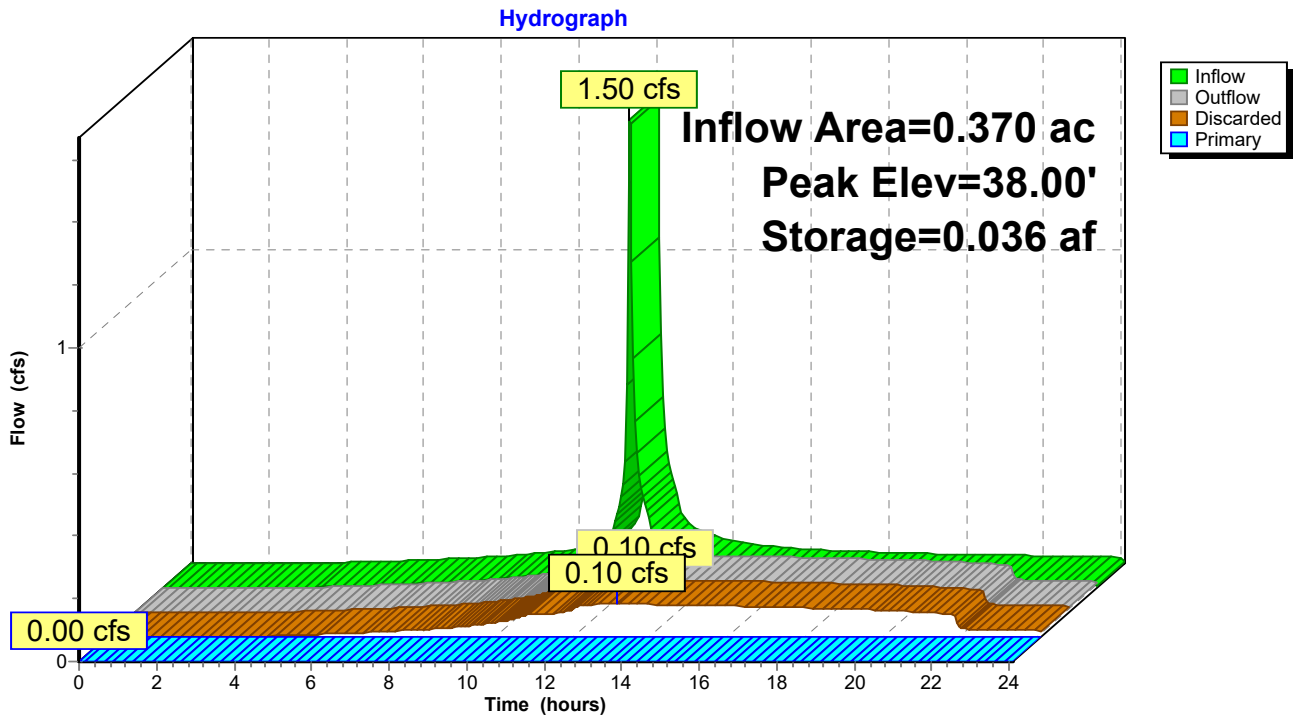
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	37.00'	1.000 in/hr Exfiltration over Wetted area Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.40'	4.0' long + 1.0 ' SideZ x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.10 cfs @ 13.16 hrs HW=38.00' (Free Discharge)
 ↑1=Exfiltration (Controls 0.10 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=37.00' (Free Discharge)
 ↑2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 22SB: Underground 22



49 Plains Road Proposed Infiltration CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

Prepared by Doane Engineering

Printed 2/9/2023

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 20: PRWS 20	Runoff Area=46,530 sf 68.29% Impervious Runoff Depth>3.65" Tc=6.0 min CN=86 Runoff=4.82 cfs 0.325 af
Subcatchment 21: PRWS 21	Runoff Area=17,460 sf 64.15% Impervious Runoff Depth>3.55" Tc=0.0 min CN=85 Runoff=2.13 cfs 0.119 af
Subcatchment 22: PRWS 22	Runoff Area=16,110 sf 78.90% Impervious Runoff Depth>4.07" Tc=6.0 min CN=90 Runoff=1.82 cfs 0.125 af
Pond 20S: Water Quality Basin	Peak Elev=34.26' Storage=9,383 cf Inflow=5.93 cfs 0.444 af Outflow=2.59 cfs 0.382 af
Pond 21SA: Water Quality Basin	Peak Elev=37.44' Storage=1,960 cf Inflow=2.13 cfs 0.119 af Outflow=2.11 cfs 0.119 af
Pond 22SB: Underground 22	Peak Elev=38.28' Storage=0.048 af Inflow=1.82 cfs 0.125 af Discarded=0.11 cfs 0.124 af Primary=0.00 cfs 0.000 af Outflow=0.11 cfs 0.124 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.569 af Average Runoff Depth = 3.72"
30.48% Pervious = 0.560 ac 69.52% Impervious = 1.278 ac

Summary for Subcatchment 20: PRWS 20

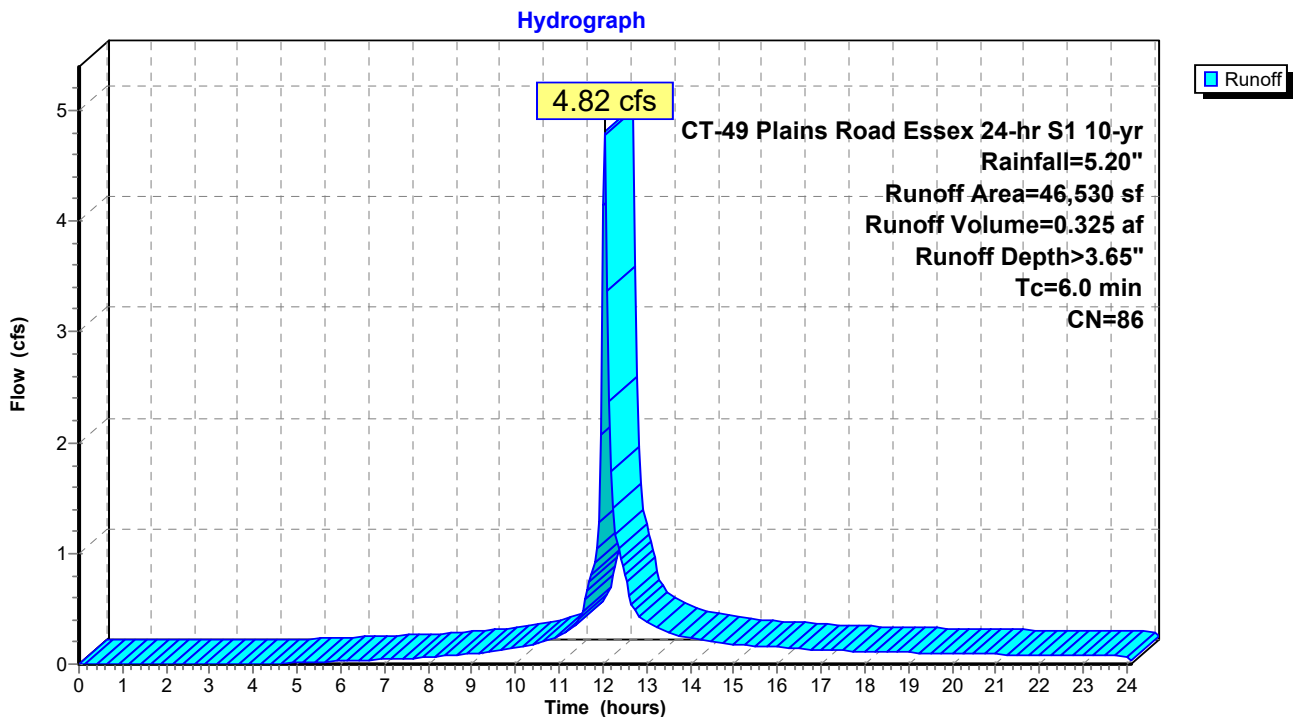
Runoff = 4.82 cfs @ 12.04 hrs, Volume= 0.325 af, Depth> 3.65"
 Routed to Pond 20S : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

Area (sf)	CN	Description
14,755	61	>75% Grass cover, Good, HSG B
29,400	98	Paved parking, HSG B
2,375	98	Unconnected roofs, HSG B
46,530	86	Weighted Average
14,755		31.71% Pervious Area
31,775		68.29% Impervious Area
2,375		7.47% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment 20: PRWS 20



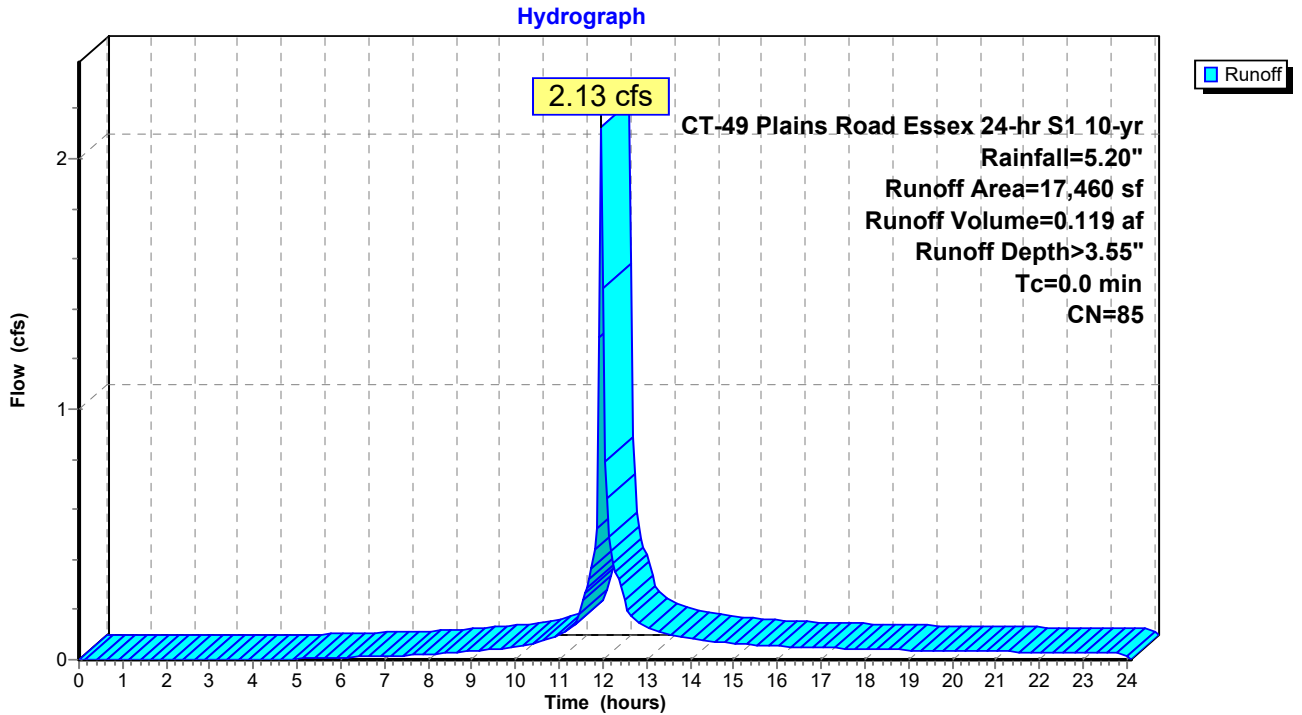
Summary for Subcatchment 21: PRWS 21

Runoff = 2.13 cfs @ 11.95 hrs, Volume= 0.119 af, Depth> 3.55"
 Routed to Pond 21SA : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

Area (sf)	CN	Description
6,260	61	>75% Grass cover, Good, HSG B
11,200	98	Paved parking, HSG B
17,460	85	Weighted Average
6,260		35.85% Pervious Area
11,200		64.15% Impervious Area

Subcatchment 21: PRWS 21



Summary for Subcatchment 22: PRWS 22

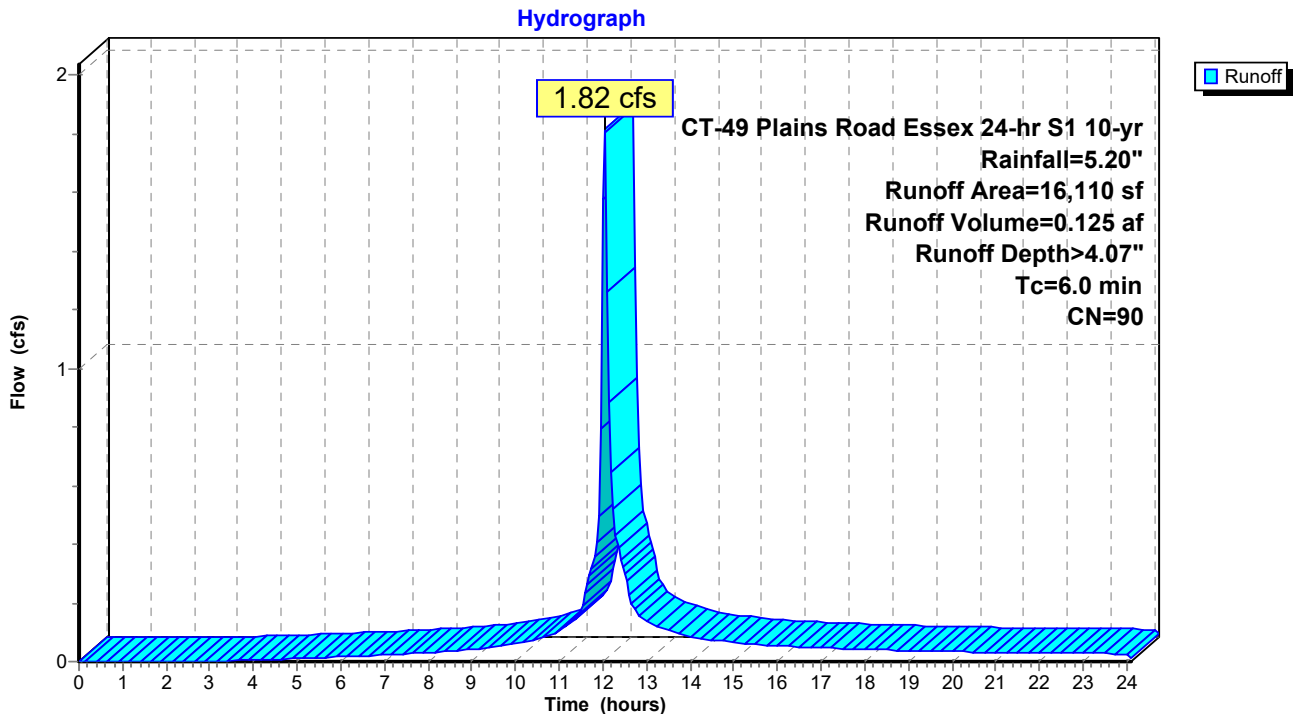
Runoff = 1.82 cfs @ 12.04 hrs, Volume= 0.125 af, Depth> 4.07"
 Routed to Pond 22SB : Underground 22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 10-yr Rainfall=5.20"

Area (sf)	CN	Description
12,500	98	Roofs, HSG B
210	98	Paved parking, HSG B
3,400	61	>75% Grass cover, Good, HSG B
16,110	90	Weighted Average
3,400		21.10% Pervious Area
12,710		78.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TR-55 TC

Subcatchment 22: PRWS 22



Summary for Pond 20S: Water Quality Basin

Inflow Area = 1.839 ac, 69.52% Impervious, Inflow Depth > 2.90" for 10-yr event
 Inflow = 5.93 cfs @ 12.02 hrs, Volume= 0.444 af
 Outflow = 2.59 cfs @ 12.17 hrs, Volume= 0.382 af, Atten= 56%, Lag= 9.1 min
 Primary = 2.59 cfs @ 12.17 hrs, Volume= 0.382 af
 Routed to nonexistent node 30

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 32.80' Surf.Area= 3,942 sf Storage= 2,955 cf
 Peak Elev= 34.26' @ 12.17 hrs Surf.Area= 4,865 sf Storage= 9,383 cf (6,428 cf above start)

Plug-Flow detention time= 214.4 min calculated for 0.314 af (71% of inflow)
 Center-of-Mass det. time= 64.8 min (882.2 - 817.3)

Volume	Invert	Avail.Storage	Storage Description			
#1	32.00'	13,158 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
32.00	3,450	299.0	0	0	3,450	
33.00	4,070	311.0	3,756	3,756	4,108	
34.00	4,700	322.0	4,381	8,137	4,746	
35.00	5,350	335.0	5,021	13,158	5,501	

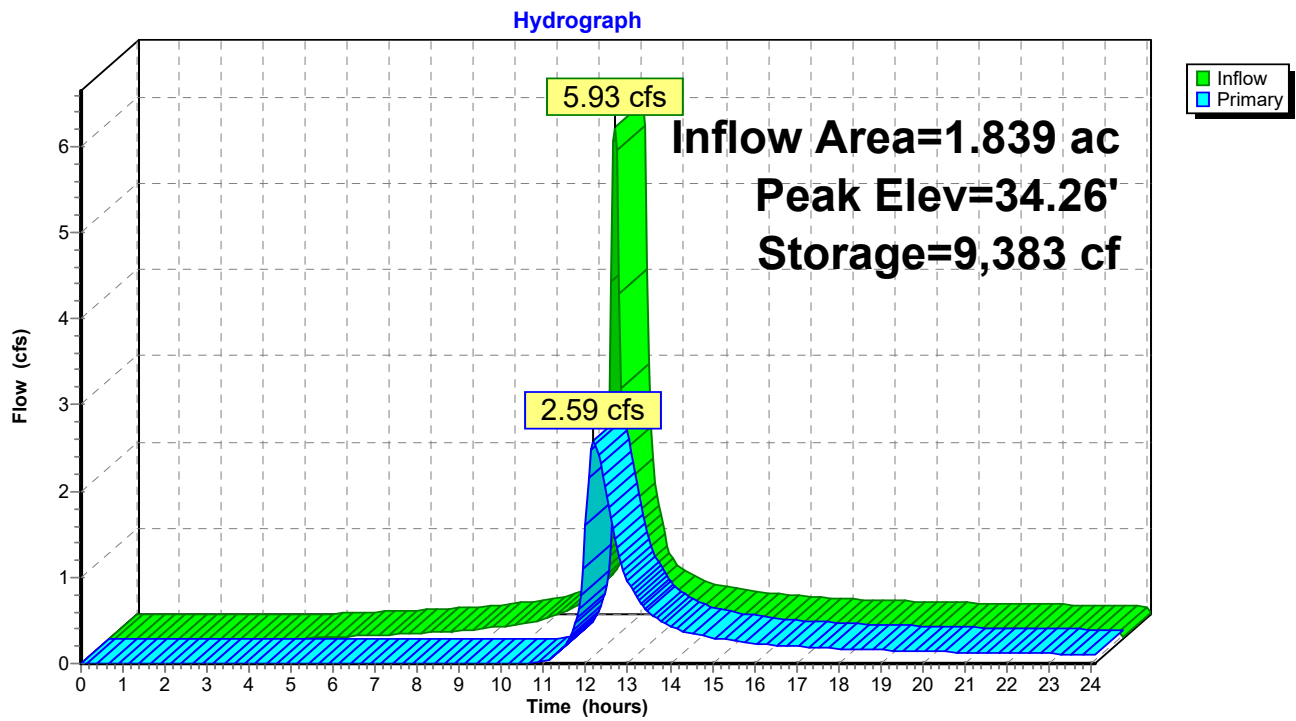
Device	Routing	Invert	Outlet Devices											
#1	Primary	34.50'	10.0' long + 0.5 ' SideZ x 3.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68 2.72 2.81 2.92 2.97 3.07 3.32											
#2	Primary	33.30'	12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads											

Primary OutFlow Max=2.57 cfs @ 12.17 hrs HW=34.26' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Orifice/Grate (Orifice Controls 2.57 cfs @ 3.33 fps)

Pond 20S: Water Quality Basin



Summary for Pond 21SA: Water Quality Basin

Inflow Area = 0.401 ac, 64.15% Impervious, Inflow Depth > 3.55" for 10-yr event
 Inflow = 2.13 cfs @ 11.95 hrs, Volume= 0.119 af
 Outflow = 2.11 cfs @ 11.96 hrs, Volume= 0.119 af, Atten= 1%, Lag= 0.4 min
 Primary = 2.11 cfs @ 11.96 hrs, Volume= 0.119 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 37.40' Surf.Area= 1,493 sf Storage= 1,899 cf
 Peak Elev= 37.44' @ 11.96 hrs Surf.Area= 1,517 sf Storage= 1,960 cf (61 cf above start)

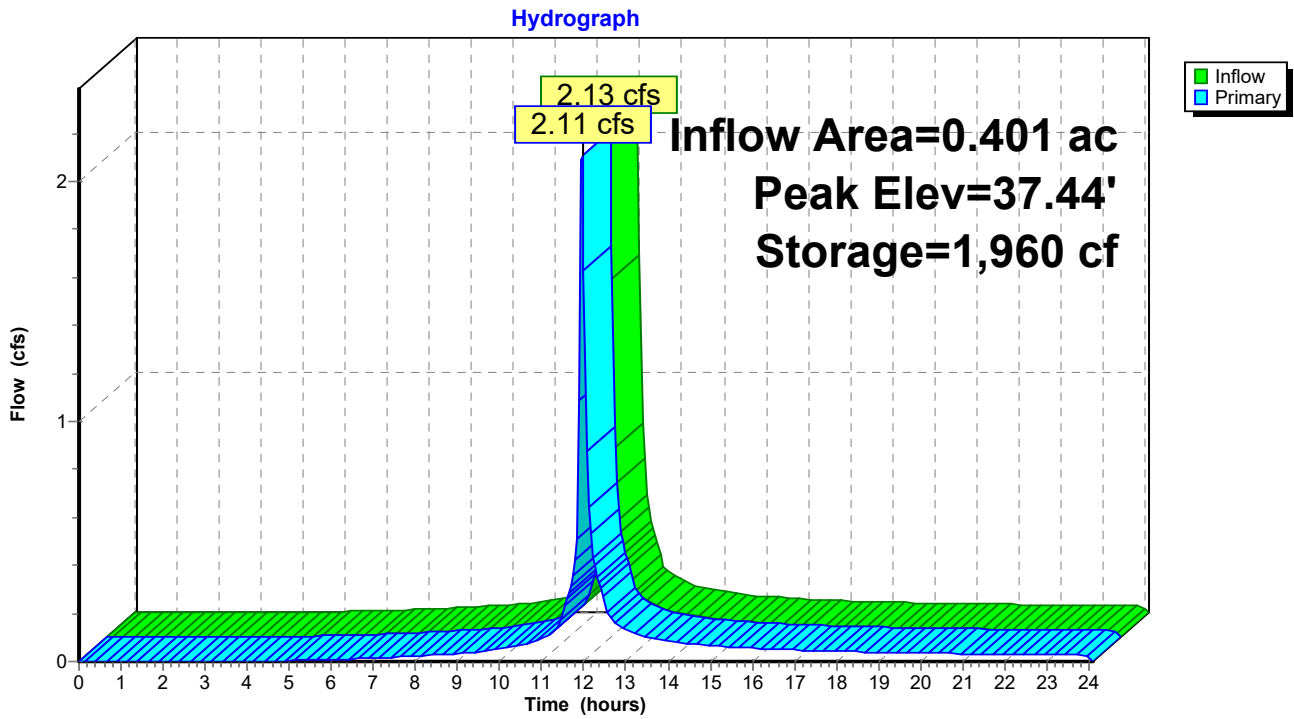
Plug-Flow detention time= 221.4 min calculated for 0.075 af (63% of inflow)
 Center-of-Mass det. time= 0.6 min (816.7 - 816.1)

Volume	Invert	Avail.Storage	Storage Description			
#1	35.00'	2,903 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
35.00	166	238.0	0	0	166	
36.00	681	264.0	394	394	1,235	
37.00	1,259	291.0	955	1,350	2,459	
37.50	1,554	298.0	702	2,052	2,819	
38.00	1,856	304.0	851	2,903	3,143	

Device	Routing	Invert	Outlet Devices
#1	Primary	37.40'	2.4" x 4.0" Horiz. Orifice/Grate X 8.00 columns X 9 rows C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.92 cfs @ 11.96 hrs HW=37.44' (Free Discharge)
 ↑1=Orifice/Grate (Weir Controls 1.92 cfs @ 0.64 fps)

Pond 21SA: Water Quality Basin



Summary for Pond 22SB: Underground 22

Inflow Area = 0.370 ac, 78.90% Impervious, Inflow Depth > 4.07" for 10-yr event
 Inflow = 1.82 cfs @ 12.04 hrs, Volume= 0.125 af
 Outflow = 0.11 cfs @ 13.34 hrs, Volume= 0.124 af, Atten= 94%, Lag= 77.9 min
 Discarded = 0.11 cfs @ 13.34 hrs, Volume= 0.124 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Peak Elev= 38.28' @ 13.34 hrs Surf.Area= 0.072 ac Storage= 0.048 af

Plug-Flow detention time= 167.3 min calculated for 0.124 af (99% of inflow)
 Center-of-Mass det. time= 161.4 min (961.4 - 800.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	37.00'	0.034 af	27.00'W x 116.39'L x 2.00'H Field A 0.144 af Overall - 0.030 af Embedded = 0.114 af x 30.0% Voids
#2A	37.50'	0.030 af	ADS_StormTech SC-160LP +Cap x 192 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 192 Chambers in 12 Rows
		0.064 af	Total Available Storage

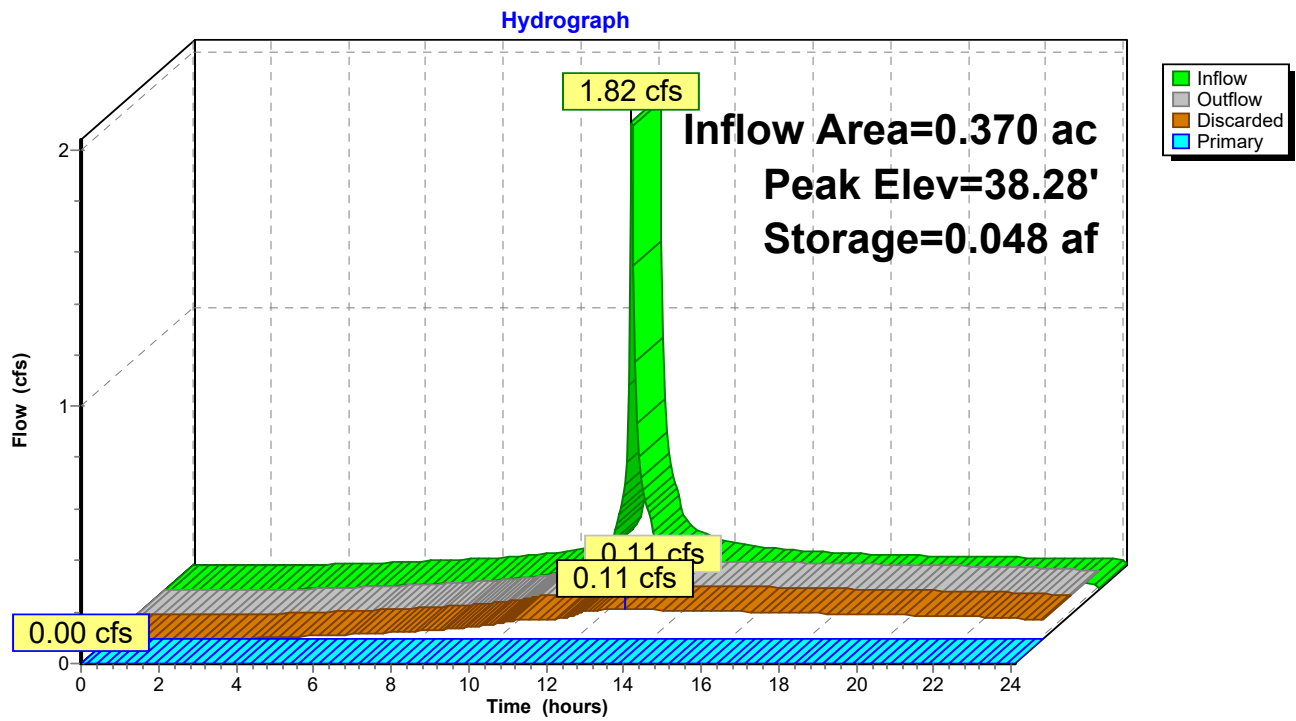
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	37.00'	1.000 in/hr Exfiltration over Wetted area Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.40'	4.0' long + 1.0 ' SideZ x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.11 cfs @ 13.34 hrs HW=38.28' (Free Discharge)
 ↑1=Exfiltration (Controls 0.11 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=37.00' (Free Discharge)
 ↑2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 22SB: Underground 22



49 Plains Road Proposed Infiltration CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

Prepared by Doane Engineering

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Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 20: PRWS 20	Runoff Area=46,530 sf 68.29% Impervious Runoff Depth>4.70" Tc=6.0 min CN=86 Runoff=6.12 cfs 0.419 af
Subcatchment 21: PRWS 21	Runoff Area=17,460 sf 64.15% Impervious Runoff Depth>4.60" Tc=0.0 min CN=85 Runoff=2.72 cfs 0.154 af
Subcatchment 22: PRWS 22	Runoff Area=16,110 sf 78.90% Impervious Runoff Depth>5.15" Tc=6.0 min CN=90 Runoff=2.26 cfs 0.159 af
Pond 20S: Water Quality Basin	Peak Elev=34.49' Storage=10,533 cf Inflow=7.51 cfs 0.586 af Outflow=3.15 cfs 0.522 af
Pond 21SA: Water Quality Basin	Peak Elev=37.45' Storage=1,971 cf Inflow=2.72 cfs 0.154 af Outflow=2.72 cfs 0.154 af
Pond 22SB: Underground 22	Peak Elev=38.50' Storage=0.053 af Inflow=2.26 cfs 0.159 af Discarded=0.12 cfs 0.136 af Primary=0.32 cfs 0.013 af Outflow=0.45 cfs 0.150 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.731 af Average Runoff Depth = 4.77"
30.48% Pervious = 0.560 ac 69.52% Impervious = 1.278 ac

Summary for Subcatchment 20: PRWS 20

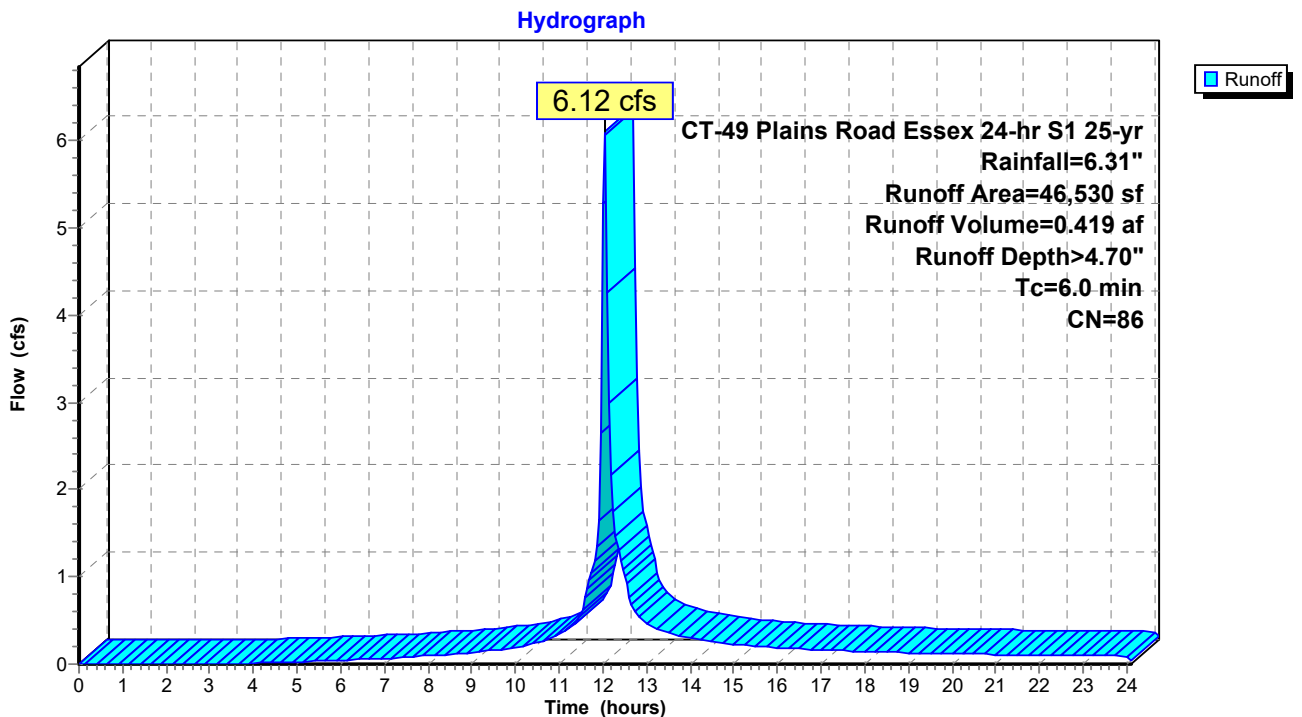
Runoff = 6.12 cfs @ 12.04 hrs, Volume= 0.419 af, Depth> 4.70"
 Routed to Pond 20S : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

Area (sf)	CN	Description
14,755	61	>75% Grass cover, Good, HSG B
29,400	98	Paved parking, HSG B
2,375	98	Unconnected roofs, HSG B
46,530	86	Weighted Average
14,755		31.71% Pervious Area
31,775		68.29% Impervious Area
2,375		7.47% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment 20: PRWS 20



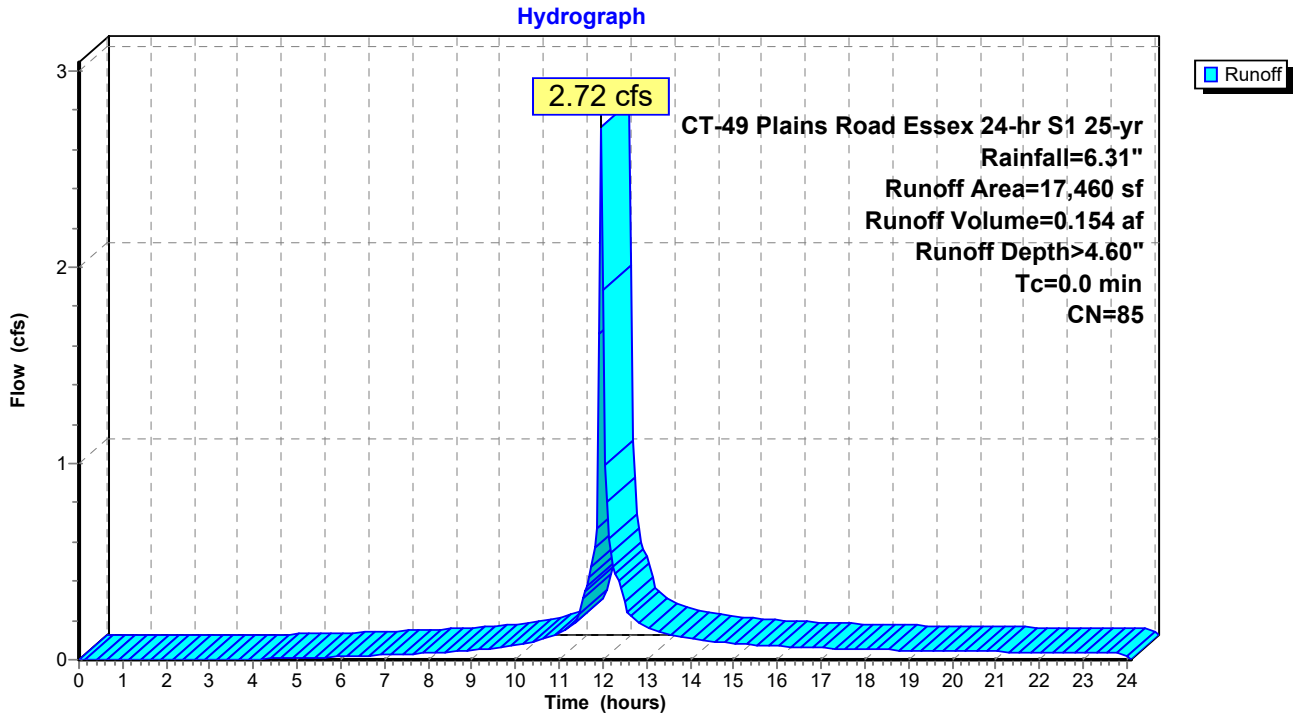
Summary for Subcatchment 21: PRWS 21

Runoff = 2.72 cfs @ 11.95 hrs, Volume= 0.154 af, Depth> 4.60"
 Routed to Pond 21SA : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

Area (sf)	CN	Description
6,260	61	>75% Grass cover, Good, HSG B
11,200	98	Paved parking, HSG B
17,460	85	Weighted Average
6,260		35.85% Pervious Area
11,200		64.15% Impervious Area

Subcatchment 21: PRWS 21



Summary for Subcatchment 22: PRWS 22

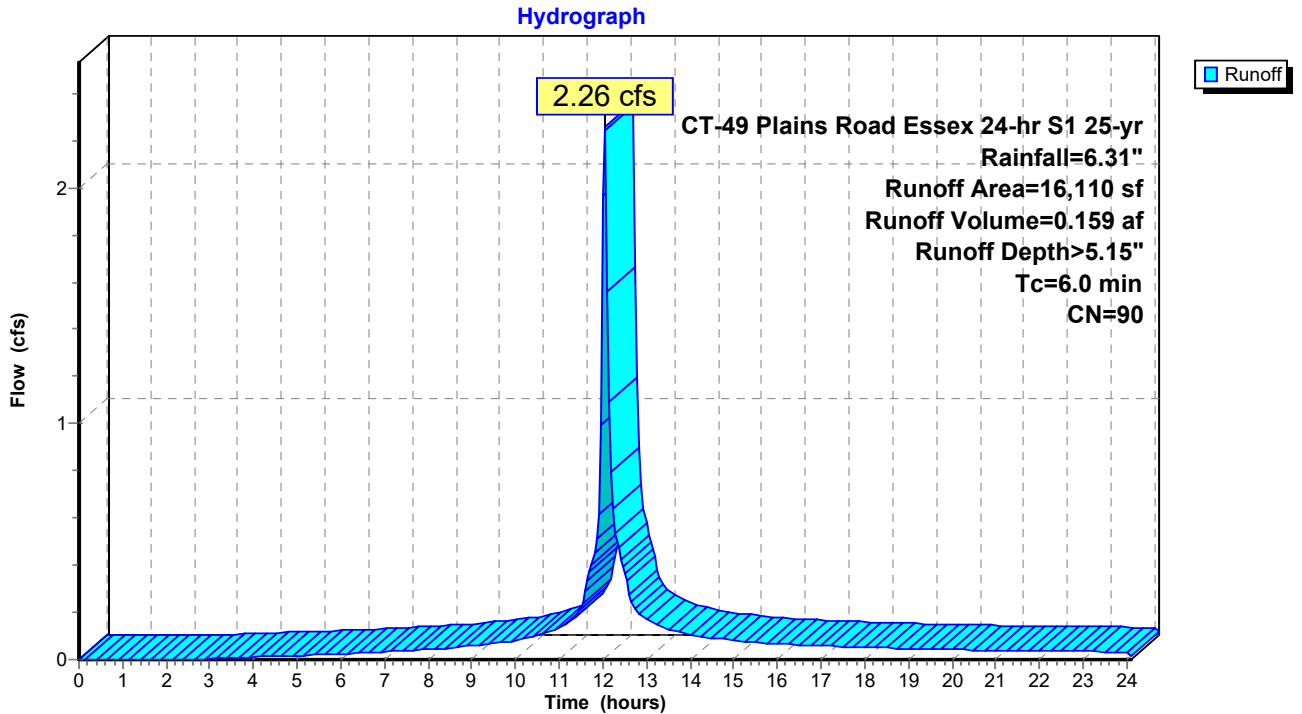
Runoff = 2.26 cfs @ 12.04 hrs, Volume= 0.159 af, Depth> 5.15"
 Routed to Pond 22SB : Underground 22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 25-yr Rainfall=6.31"

Area (sf)	CN	Description
12,500	98	Roofs, HSG B
210	98	Paved parking, HSG B
3,400	61	>75% Grass cover, Good, HSG B
16,110	90	Weighted Average
3,400		21.10% Pervious Area
12,710		78.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TR-55 TC

Subcatchment 22: PRWS 22



Summary for Pond 20S: Water Quality Basin

Inflow Area = 1.839 ac, 69.52% Impervious, Inflow Depth > 3.82" for 25-yr event
 Inflow = 7.51 cfs @ 12.02 hrs, Volume= 0.586 af
 Outflow = 3.15 cfs @ 12.18 hrs, Volume= 0.522 af, Atten= 58%, Lag= 9.4 min
 Primary = 3.15 cfs @ 12.18 hrs, Volume= 0.522 af
 Routed to nonexistent node 30

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 32.80' Surf.Area= 3,942 sf Storage= 2,955 cf
 Peak Elev= 34.49' @ 12.18 hrs Surf.Area= 5,015 sf Storage= 10,533 cf (7,578 cf above start)

Plug-Flow detention time= 181.9 min calculated for 0.454 af (78% of inflow)
 Center-of-Mass det. time= 59.0 min (866.2 - 807.3)

Volume	Invert	Avail.Storage	Storage Description			
#1	32.00'	13,158 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
32.00	3,450	299.0	0	0	3,450	
33.00	4,070	311.0	3,756	3,756	4,108	
34.00	4,700	322.0	4,381	8,137	4,746	
35.00	5,350	335.0	5,021	13,158	5,501	

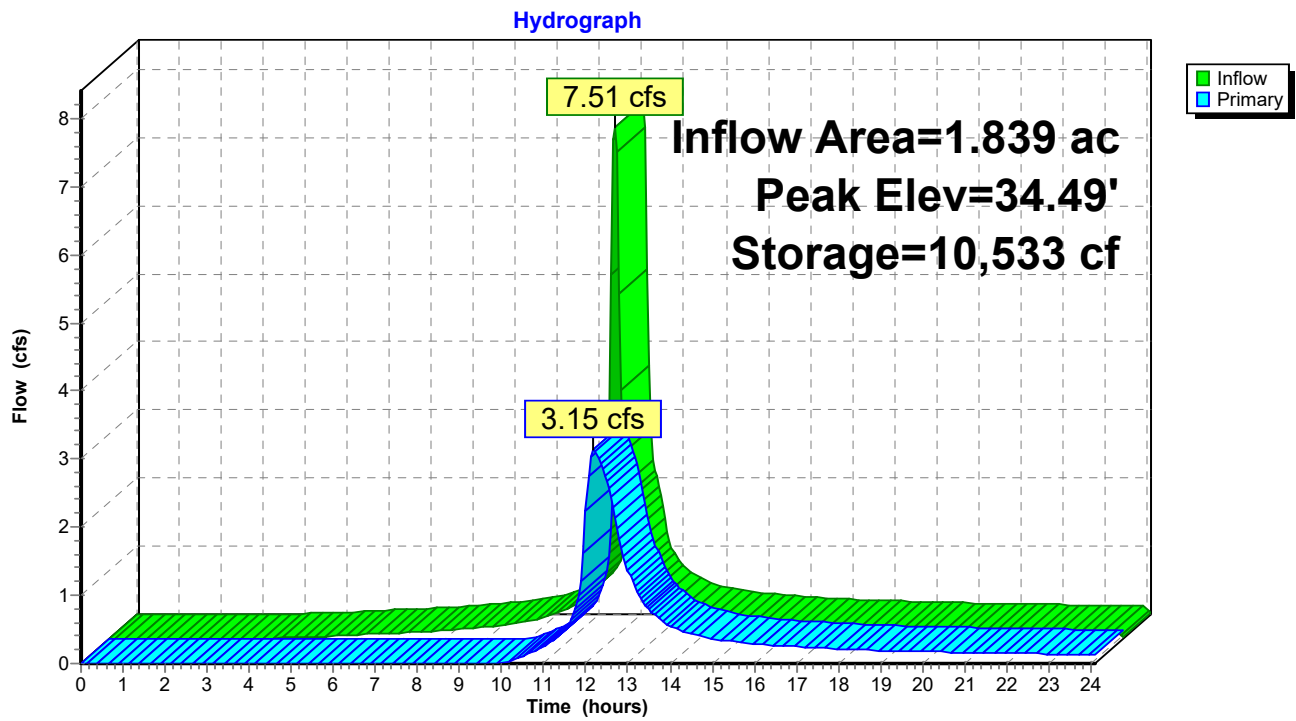
Device	Routing	Invert	Outlet Devices											
#1	Primary	34.50'	10.0' long + 0.5 ' SideZ x 3.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68 2.72 2.81 2.92 2.97 3.07 3.32											
#2	Primary	33.30'	12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads											

Primary OutFlow Max=3.14 cfs @ 12.18 hrs HW=34.49' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Orifice/Grate (Orifice Controls 3.14 cfs @ 4.00 fps)

Pond 20S: Water Quality Basin



Summary for Pond 21SA: Water Quality Basin

Inflow Area = 0.401 ac, 64.15% Impervious, Inflow Depth > 4.60" for 25-yr event
 Inflow = 2.72 cfs @ 11.95 hrs, Volume= 0.154 af
 Outflow = 2.72 cfs @ 11.96 hrs, Volume= 0.154 af, Atten= 0%, Lag= 0.3 min
 Primary = 2.72 cfs @ 11.96 hrs, Volume= 0.154 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 37.40' Surf.Area= 1,493 sf Storage= 1,899 cf
 Peak Elev= 37.45' @ 11.96 hrs Surf.Area= 1,522 sf Storage= 1,971 cf (72 cf above start)

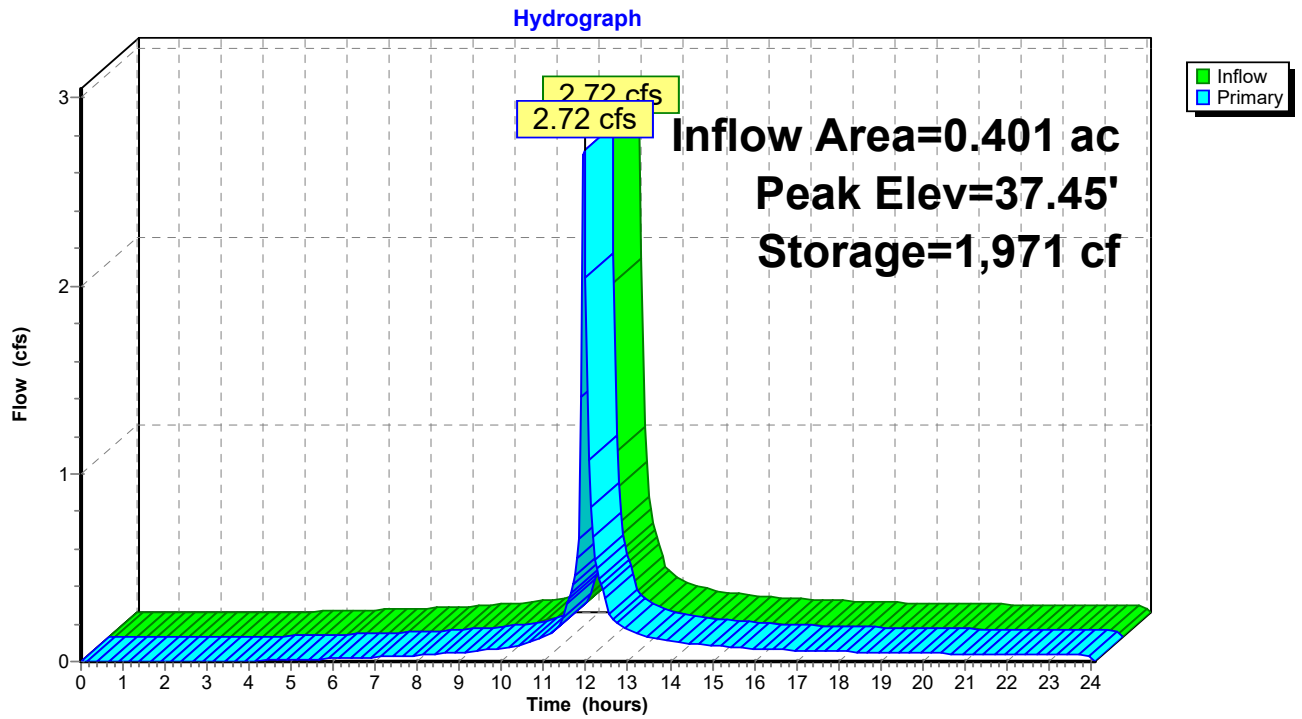
Plug-Flow detention time= 187.9 min calculated for 0.110 af (72% of inflow)
 Center-of-Mass det. time= 0.6 min (807.6 - 807.0)

Volume	Invert	Avail.Storage	Storage Description			
#1	35.00'	2,903 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
35.00	166	238.0	0	0	166	
36.00	681	264.0	394	394	1,235	
37.00	1,259	291.0	955	1,350	2,459	
37.50	1,554	298.0	702	2,052	2,819	
38.00	1,856	304.0	851	2,903	3,143	

Device	Routing	Invert	Outlet Devices
#1	Primary	37.40'	2.4" x 4.0" Horiz. Orifice/Grate X 8.00 columns X 9 rows C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.49 cfs @ 11.96 hrs HW=37.45' (Free Discharge)
 ↑1=Orifice/Grate (Weir Controls 2.49 cfs @ 0.70 fps)

Pond 21SA: Water Quality Basin



Summary for Pond 22SB: Underground 22

Inflow Area = 0.370 ac, 78.90% Impervious, Inflow Depth > 5.15" for 25-yr event
 Inflow = 2.26 cfs @ 12.04 hrs, Volume= 0.159 af
 Outflow = 0.45 cfs @ 12.39 hrs, Volume= 0.150 af, Atten= 80%, Lag= 21.0 min
 Discarded = 0.12 cfs @ 12.39 hrs, Volume= 0.136 af
 Primary = 0.32 cfs @ 12.39 hrs, Volume= 0.013 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Peak Elev= 38.50' @ 12.39 hrs Surf.Area= 0.072 ac Storage= 0.053 af

Plug-Flow detention time= 168.8 min calculated for 0.149 af (94% of inflow)
 Center-of-Mass det. time= 135.8 min (928.1 - 792.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	37.00'	0.034 af	27.00'W x 116.39'L x 2.00'H Field A 0.144 af Overall - 0.030 af Embedded = 0.114 af x 30.0% Voids
#2A	37.50'	0.030 af	ADS_StormTech SC-160LP +Cap x 192 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 192 Chambers in 12 Rows
		0.064 af	Total Available Storage

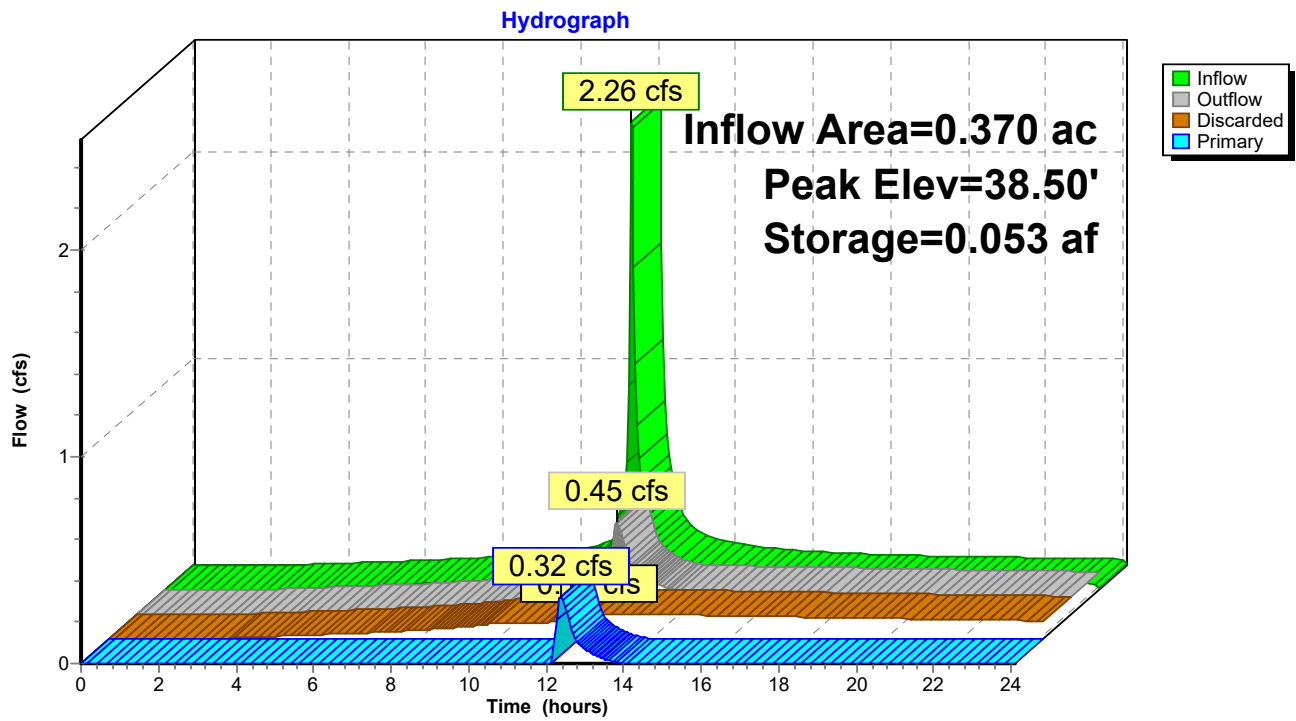
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	37.00'	1.000 in/hr Exfiltration over Wetted area Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.40'	4.0' long + 1.0 ' SideZ x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.12 cfs @ 12.39 hrs HW=38.49' (Free Discharge)
 ↑1=Exfiltration (Controls 0.12 cfs)

Primary OutFlow Max=0.32 cfs @ 12.39 hrs HW=38.49' (Free Discharge)
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 0.32 cfs @ 0.83 fps)

Pond 22SB: Underground 22



Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 20: PRWS 20	Runoff Area=46,530 sf 68.29% Impervious Runoff Depth>5.49" Tc=6.0 min CN=86 Runoff=7.09 cfs 0.489 af
Subcatchment 21: PRWS 21	Runoff Area=17,460 sf 64.15% Impervious Runoff Depth>5.38" Tc=0.0 min CN=85 Runoff=3.16 cfs 0.180 af
Subcatchment 22: PRWS 22	Runoff Area=16,110 sf 78.90% Impervious Runoff Depth>5.95" Tc=6.0 min CN=90 Runoff=2.60 cfs 0.183 af
Pond 20S: Water Quality Basin	Peak Elev=34.63' Storage=11,215 cf Inflow=8.71 cfs 0.696 af Outflow=4.57 cfs 0.631 af
Pond 21SA: Water Quality Basin	Peak Elev=37.45' Storage=1,980 cf Inflow=3.16 cfs 0.180 af Outflow=3.15 cfs 0.180 af
Pond 22SB: Underground 22	Peak Elev=38.58' Storage=0.055 af Inflow=2.60 cfs 0.183 af Discarded=0.12 cfs 0.143 af Primary=0.86 cfs 0.028 af Outflow=0.99 cfs 0.170 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.852 af Average Runoff Depth = 5.56"
30.48% Pervious = 0.560 ac 69.52% Impervious = 1.278 ac

Summary for Subcatchment 20: PRWS 20

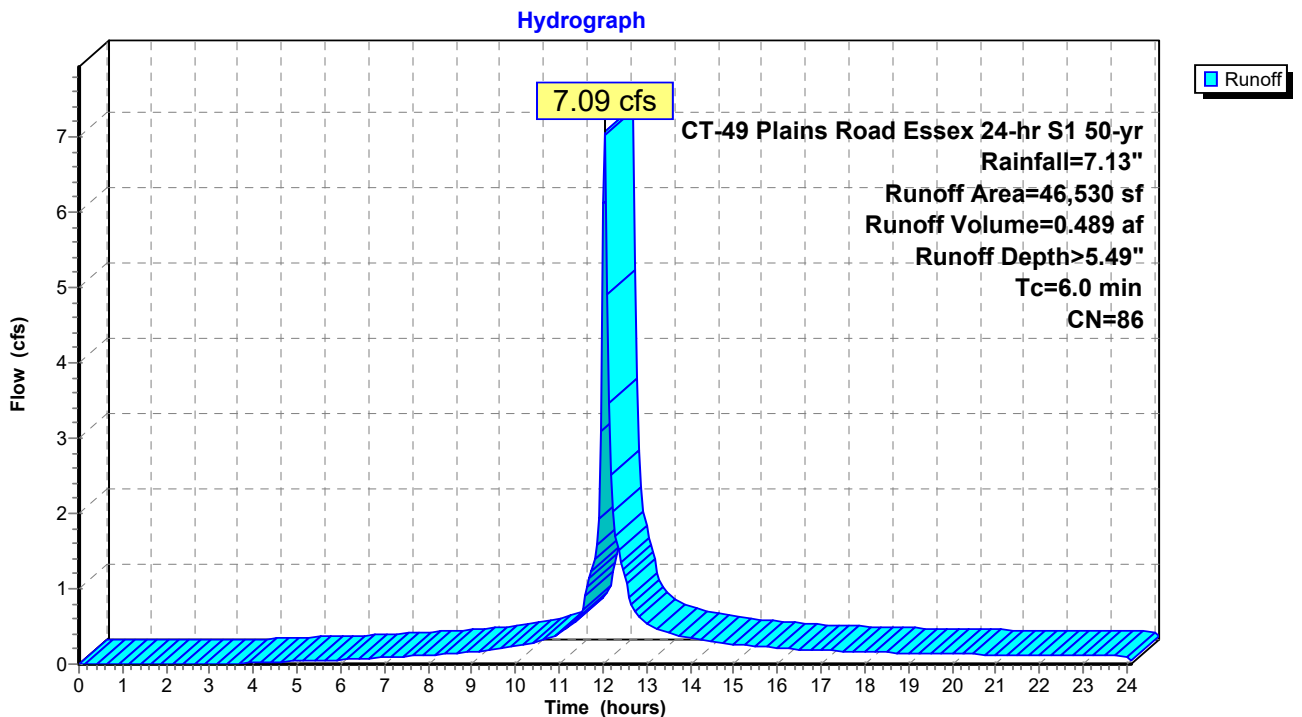
Runoff = 7.09 cfs @ 12.04 hrs, Volume= 0.489 af, Depth> 5.49"
 Routed to Pond 20S : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 50-yr Rainfall=7.13"

Area (sf)	CN	Description
14,755	61	>75% Grass cover, Good, HSG B
29,400	98	Paved parking, HSG B
2,375	98	Unconnected roofs, HSG B
46,530	86	Weighted Average
14,755		31.71% Pervious Area
31,775		68.29% Impervious Area
2,375		7.47% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment 20: PRWS 20



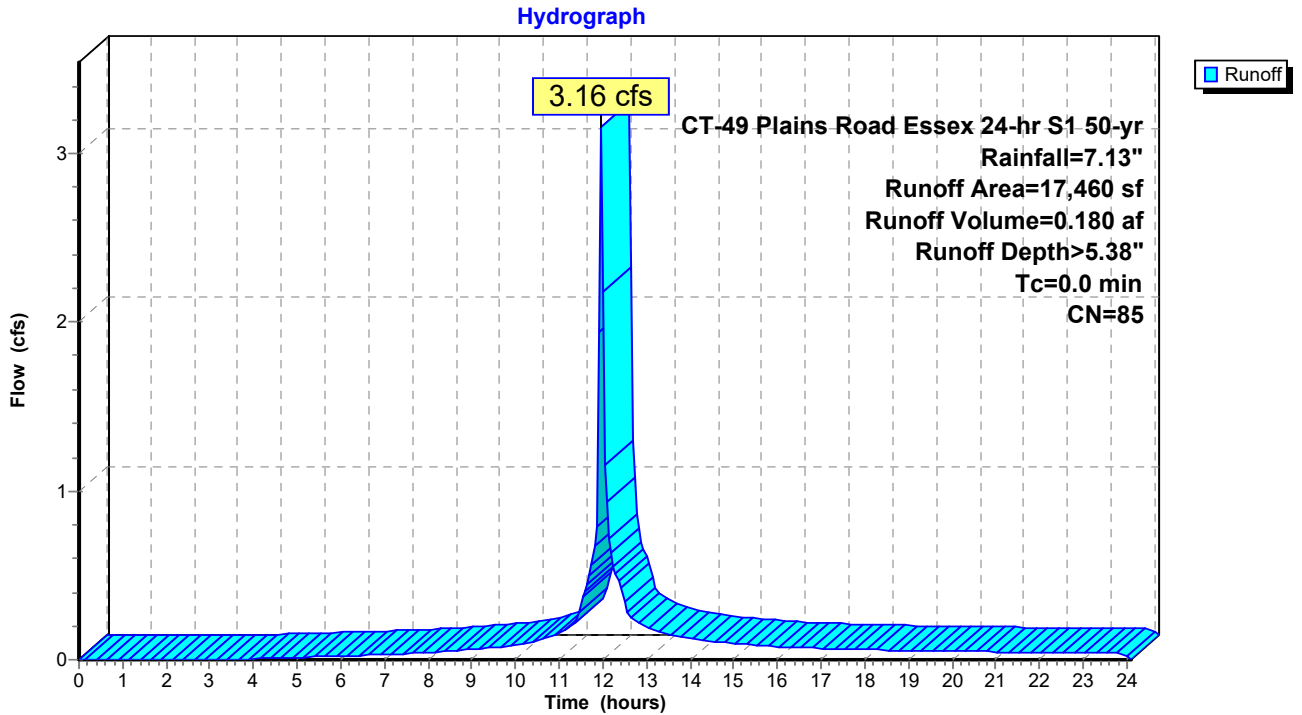
Summary for Subcatchment 21: PRWS 21

Runoff = 3.16 cfs @ 11.95 hrs, Volume= 0.180 af, Depth> 5.38"
 Routed to Pond 21SA : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 50-yr Rainfall=7.13"

Area (sf)	CN	Description
6,260	61	>75% Grass cover, Good, HSG B
11,200	98	Paved parking, HSG B
17,460	85	Weighted Average
6,260		35.85% Pervious Area
11,200		64.15% Impervious Area

Subcatchment 21: PRWS 21



Summary for Subcatchment 22: PRWS 22

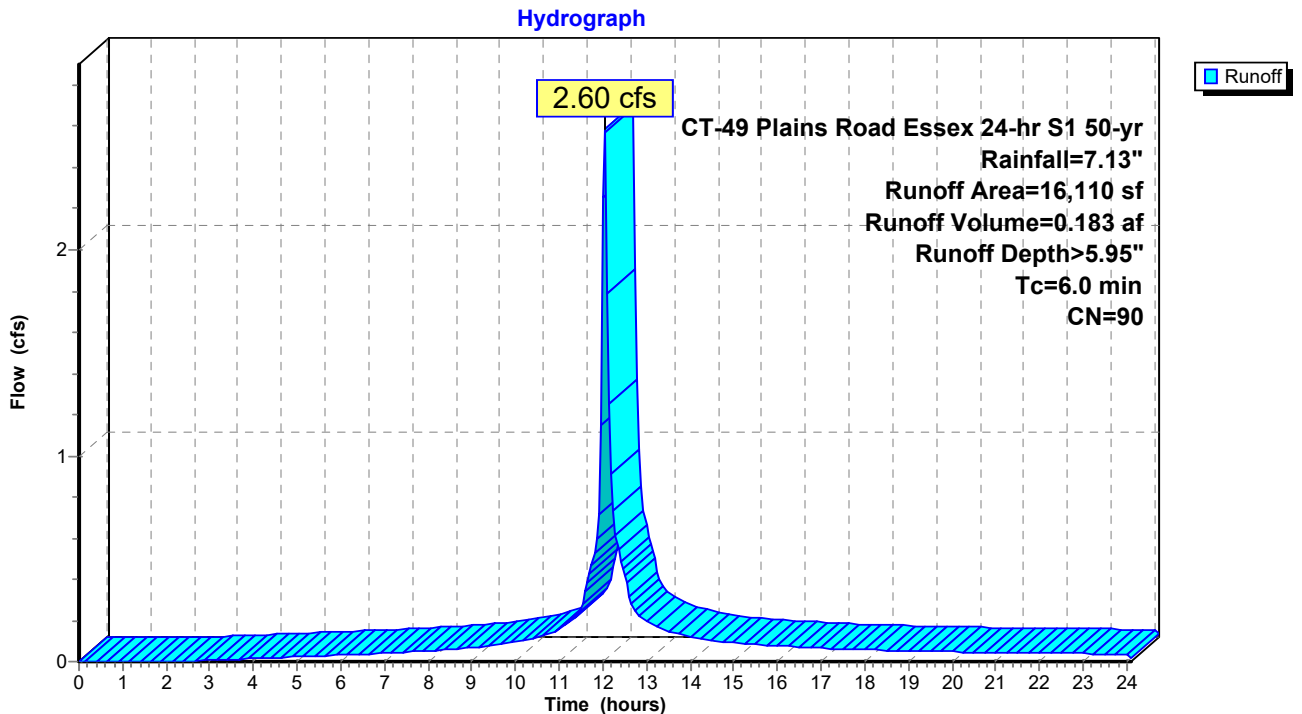
Runoff = 2.60 cfs @ 12.04 hrs, Volume= 0.183 af, Depth> 5.95"
 Routed to Pond 22SB : Underground 22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 50-yr Rainfall=7.13"

Area (sf)	CN	Description
12,500	98	Roofs, HSG B
210	98	Paved parking, HSG B
3,400	61	>75% Grass cover, Good, HSG B
16,110	90	Weighted Average
3,400		21.10% Pervious Area
12,710		78.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TR-55 TC

Subcatchment 22: PRWS 22



Summary for Pond 20S: Water Quality Basin

Inflow Area = 1.839 ac, 69.52% Impervious, Inflow Depth > 4.54" for 50-yr event
 Inflow = 8.71 cfs @ 12.02 hrs, Volume= 0.696 af
 Outflow = 4.57 cfs @ 12.17 hrs, Volume= 0.631 af, Atten= 48%, Lag= 8.9 min
 Primary = 4.57 cfs @ 12.17 hrs, Volume= 0.631 af
 Routed to nonexistent node 30

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 32.80' Surf.Area= 3,942 sf Storage= 2,955 cf
 Peak Elev= 34.63' @ 12.17 hrs Surf.Area= 5,103 sf Storage= 11,215 cf (8,260 cf above start)

Plug-Flow detention time= 163.5 min calculated for 0.562 af (81% of inflow)
 Center-of-Mass det. time= 54.9 min (855.6 - 800.7)

Volume	Invert	Avail.Storage	Storage Description			
#1	32.00'	13,158 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
32.00	3,450	299.0	0	0	3,450	
33.00	4,070	311.0	3,756	3,756	4,108	
34.00	4,700	322.0	4,381	8,137	4,746	
35.00	5,350	335.0	5,021	13,158	5,501	

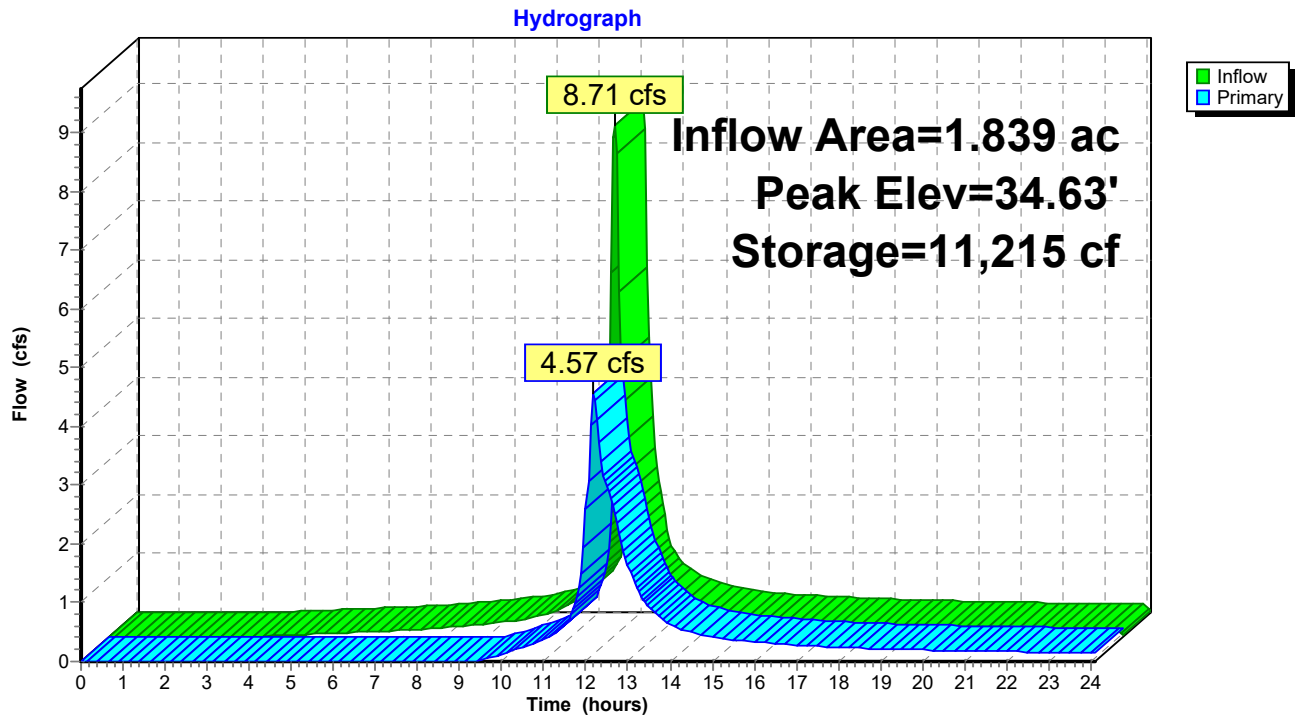
Device	Routing	Invert	Outlet Devices											
#1	Primary	34.50'	10.0' long + 0.5 ' SideZ x 3.0' breadth Broad-Crested Rectangular Weir											
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00											
			2.50 3.00 3.50 4.00 4.50											
			Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68											
			2.72 2.81 2.92 2.97 3.07 3.32											
#2	Primary	33.30'	12.0" Vert. Orifice/Grate C= 0.600											
			Limited to weir flow at low heads											

Primary OutFlow Max=4.49 cfs @ 12.17 hrs HW=34.62' (Free Discharge)

1=**Broad-Crested Rectangular Weir** (Weir Controls 1.06 cfs @ 0.86 fps)

2=**Orifice/Grate** (Orifice Controls 3.43 cfs @ 4.37 fps)

Pond 20S: Water Quality Basin



Summary for Pond 21SA: Water Quality Basin

Inflow Area = 0.401 ac, 64.15% Impervious, Inflow Depth > 5.38" for 50-yr event
 Inflow = 3.16 cfs @ 11.95 hrs, Volume= 0.180 af
 Outflow = 3.15 cfs @ 11.96 hrs, Volume= 0.180 af, Atten= 1%, Lag= 0.3 min
 Primary = 3.15 cfs @ 11.96 hrs, Volume= 0.180 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 37.40' Surf.Area= 1,493 sf Storage= 1,899 cf
 Peak Elev= 37.45' @ 11.96 hrs Surf.Area= 1,525 sf Storage= 1,980 cf (80 cf above start)

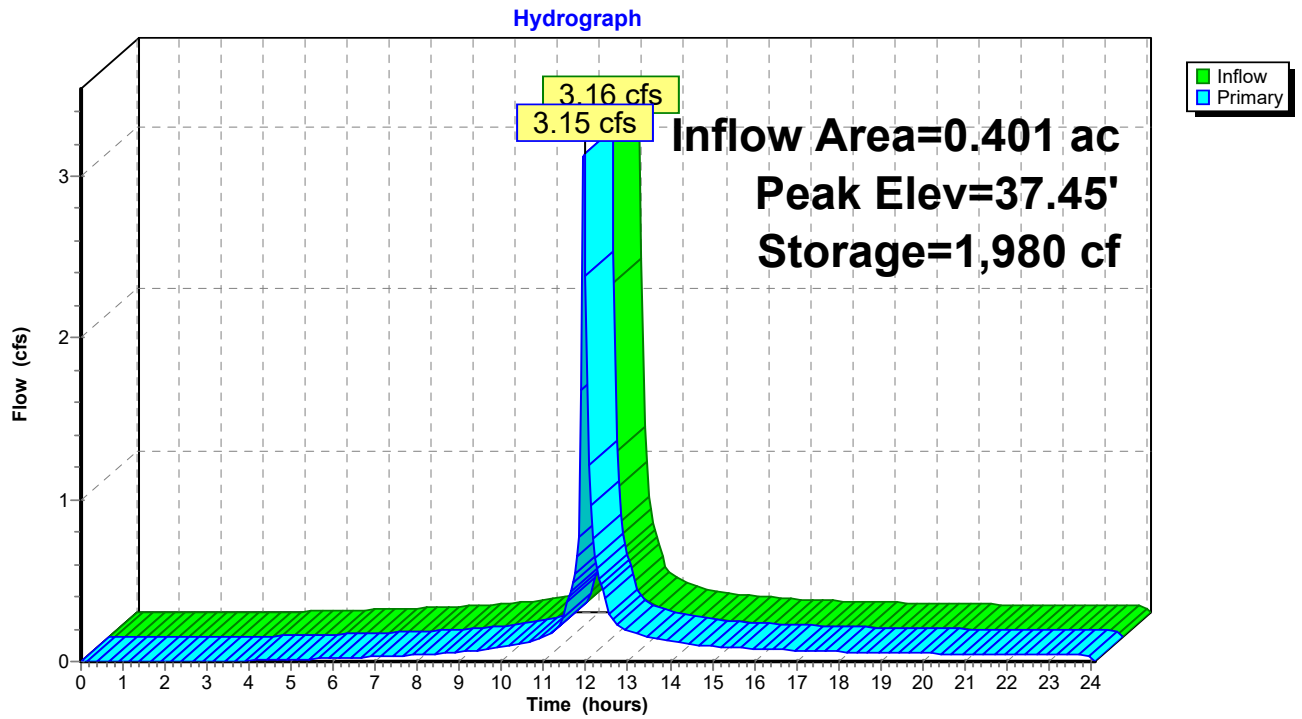
Plug-Flow detention time= 169.8 min calculated for 0.136 af (76% of inflow)
 Center-of-Mass det. time= 0.6 min (801.8 - 801.2)

Volume	Invert	Avail.Storage	Storage Description			
#1	35.00'	2,903 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
35.00	166	238.0	0	0	166	
36.00	681	264.0	394	394	1,235	
37.00	1,259	291.0	955	1,350	2,459	
37.50	1,554	298.0	702	2,052	2,819	
38.00	1,856	304.0	851	2,903	3,143	

Device	Routing	Invert	Outlet Devices
#1	Primary	37.40'	2.4" x 4.0" Horiz. Orifice/Grate X 8.00 columns X 9 rows C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.92 cfs @ 11.96 hrs HW=37.45' (Free Discharge)
 ↑1=Orifice/Grate (Weir Controls 2.92 cfs @ 0.74 fps)

Pond 21SA: Water Quality Basin



Summary for Pond 22SB: Underground 22

Inflow Area = 0.370 ac, 78.90% Impervious, Inflow Depth > 5.95" for 50-yr event
 Inflow = 2.60 cfs @ 12.04 hrs, Volume= 0.183 af
 Outflow = 0.99 cfs @ 12.21 hrs, Volume= 0.170 af, Atten= 62%, Lag= 10.5 min
 Discarded = 0.12 cfs @ 12.21 hrs, Volume= 0.143 af
 Primary = 0.86 cfs @ 12.21 hrs, Volume= 0.028 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Peak Elev= 38.58' @ 12.21 hrs Surf.Area= 0.072 ac Storage= 0.055 af

Plug-Flow detention time= 156.6 min calculated for 0.170 af (93% of inflow)
 Center-of-Mass det. time= 115.6 min (902.9 - 787.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	37.00'	0.034 af	27.00'W x 116.39'L x 2.00'H Field A 0.144 af Overall - 0.030 af Embedded = 0.114 af x 30.0% Voids
#2A	37.50'	0.030 af	ADS_StormTech SC-160LP +Cap x 192 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 192 Chambers in 12 Rows
		0.064 af	Total Available Storage

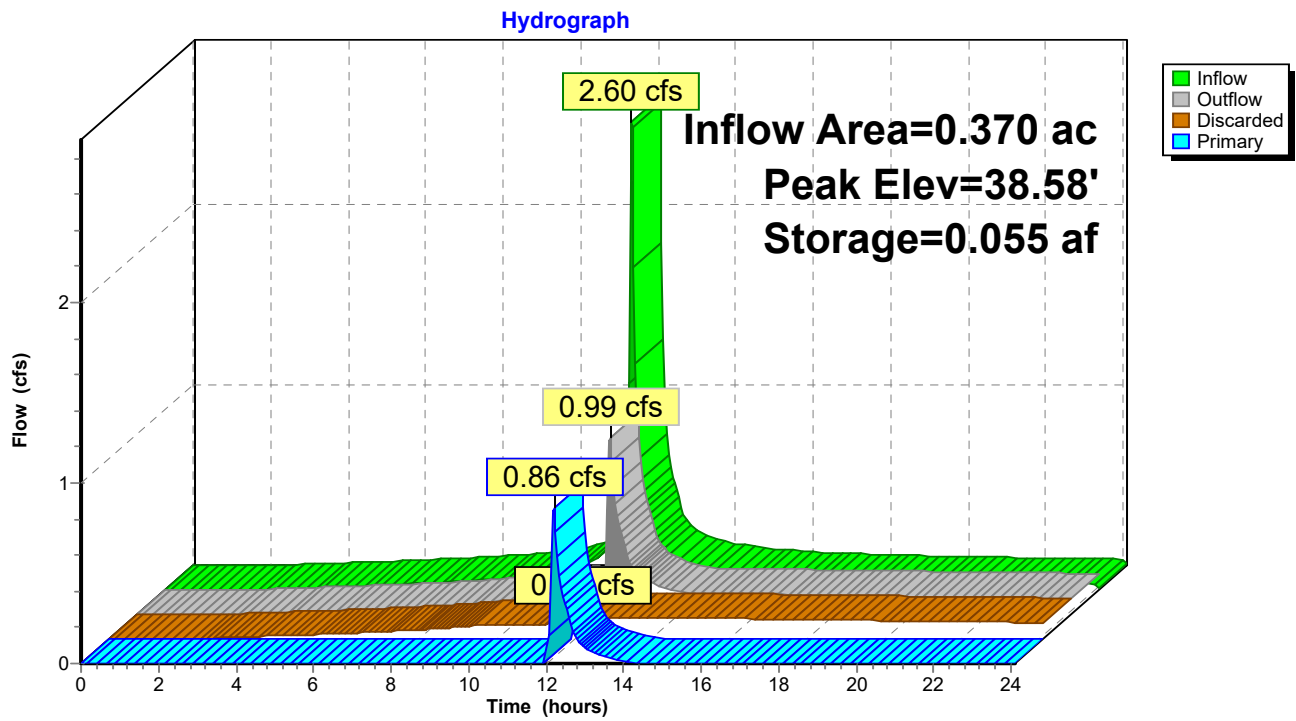
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	37.00'	1.000 in/hr Exfiltration over Wetted area Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.40'	4.0' long + 1.0 ' SideZ x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.12 cfs @ 12.21 hrs HW=38.57' (Free Discharge)
 ↑1=Exfiltration (Controls 0.12 cfs)

Primary OutFlow Max=0.81 cfs @ 12.21 hrs HW=38.57' (Free Discharge)
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 0.81 cfs @ 1.11 fps)

Pond 22SB: Underground 22



Time span=0.00-24.10 hrs, dt=0.05 hrs, 483 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment20: PRWS 20	Runoff Area=46,530 sf 68.29% Impervious Runoff Depth>6.34" Tc=6.0 min CN=86 Runoff=8.10 cfs 0.564 af
Subcatchment21: PRWS 21	Runoff Area=17,460 sf 64.15% Impervious Runoff Depth>6.22" Tc=0.0 min CN=85 Runoff=3.63 cfs 0.208 af
Subcatchment22: PRWS 22	Runoff Area=16,110 sf 78.90% Impervious Runoff Depth>6.81" Tc=6.0 min CN=90 Runoff=2.94 cfs 0.210 af
Pond 20S: Water Quality Basin	Peak Elev=34.74' Storage=11,800 cf Inflow=9.97 cfs 0.816 af Outflow=6.64 cfs 0.750 af
Pond 21SA: Water Quality Basin	Peak Elev=37.46' Storage=1,988 cf Inflow=3.63 cfs 0.208 af Outflow=3.59 cfs 0.208 af
Pond 22SB: Underground 22	Peak Elev=38.67' Storage=0.057 af Inflow=2.94 cfs 0.210 af Discarded=0.13 cfs 0.149 af Primary=1.55 cfs 0.044 af Outflow=1.68 cfs 0.193 af

Total Runoff Area = 1.839 ac Runoff Volume = 0.982 af Average Runoff Depth = 6.41"
30.48% Pervious = 0.560 ac 69.52% Impervious = 1.278 ac

Summary for Subcatchment 20: PRWS 20

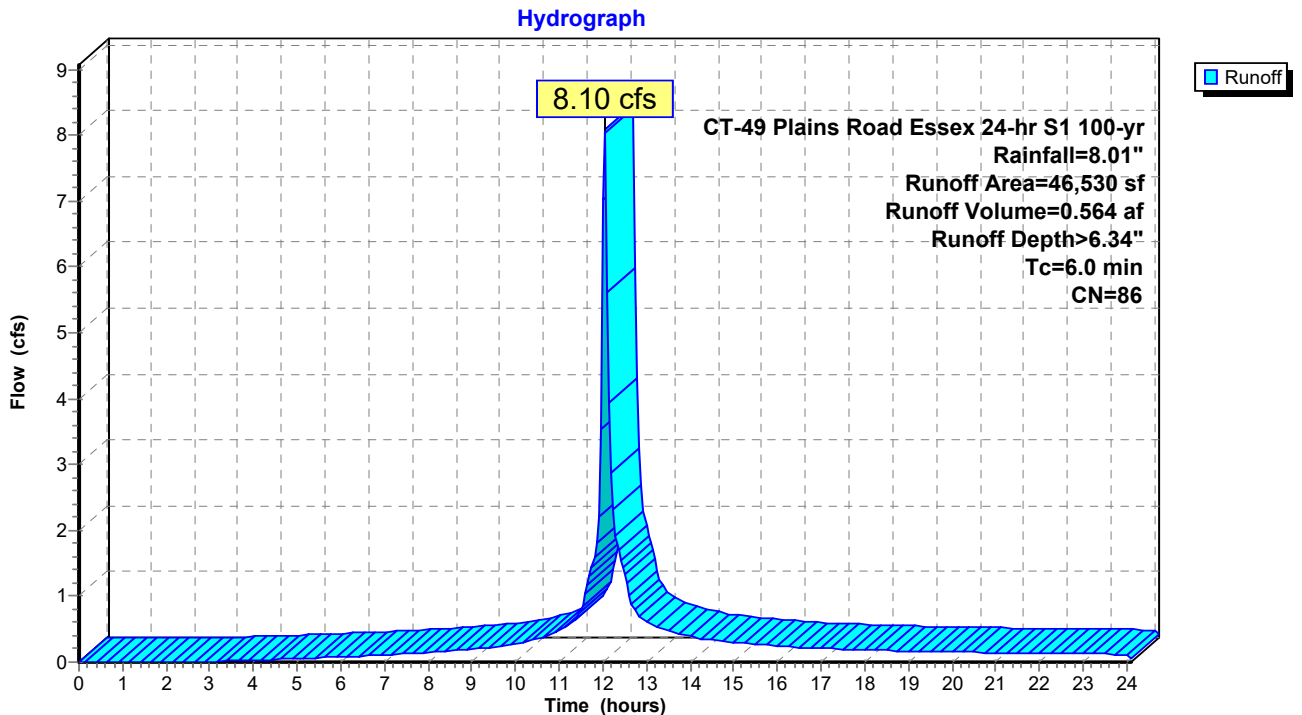
Runoff = 8.10 cfs @ 12.04 hrs, Volume= 0.564 af, Depth> 6.34"
 Routed to Pond 20S : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 100-yr Rainfall=8.01"

Area (sf)	CN	Description
14,755	61	>75% Grass cover, Good, HSG B
29,400	98	Paved parking, HSG B
2,375	98	Unconnected roofs, HSG B
46,530	86	Weighted Average
14,755		31.71% Pervious Area
31,775		68.29% Impervious Area
2,375		7.47% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment 20: PRWS 20



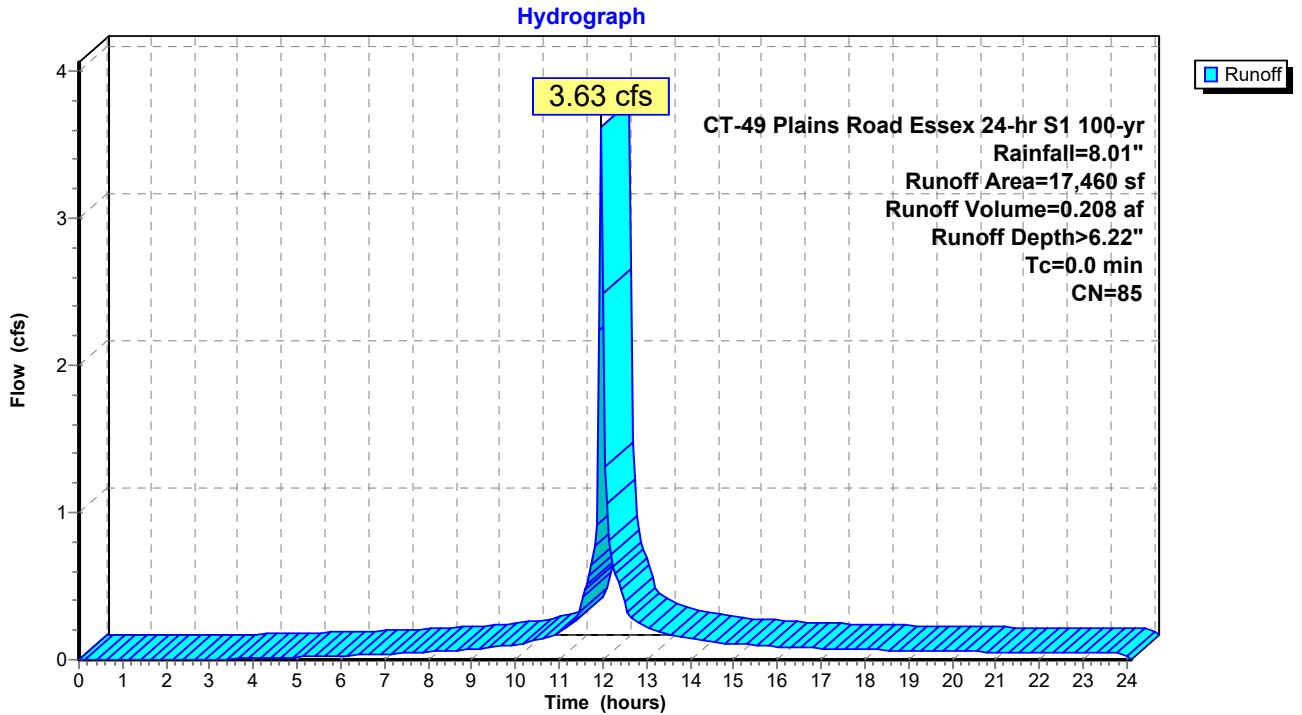
Summary for Subcatchment 21: PRWS 21

Runoff = 3.63 cfs @ 11.95 hrs, Volume= 0.208 af, Depth> 6.22"
 Routed to Pond 21SA : Water Quality Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 100-yr Rainfall=8.01"

Area (sf)	CN	Description
6,260	61	>75% Grass cover, Good, HSG B
11,200	98	Paved parking, HSG B
17,460	85	Weighted Average
6,260		35.85% Pervious Area
11,200		64.15% Impervious Area

Subcatchment 21: PRWS 21



Summary for Subcatchment 22: PRWS 22

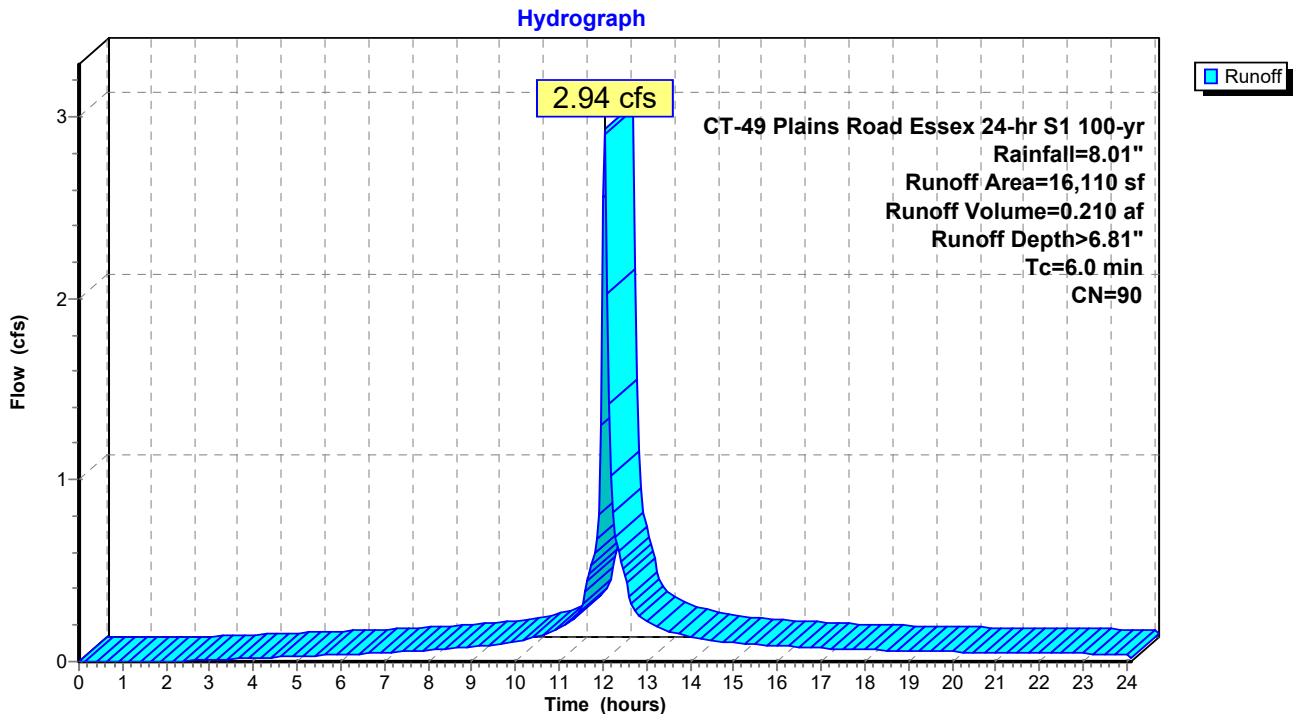
Runoff = 2.94 cfs @ 12.04 hrs, Volume= 0.210 af, Depth> 6.81"
 Routed to Pond 22SB : Underground 22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 CT-49 Plains Road Essex 24-hr S1 100-yr Rainfall=8.01"

Area (sf)	CN	Description
12,500	98	Roofs, HSG B
210	98	Paved parking, HSG B
3,400	61	>75% Grass cover, Good, HSG B
16,110	90	Weighted Average
3,400		21.10% Pervious Area
12,710		78.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TR-55 TC

Subcatchment 22: PRWS 22



Summary for Pond 20S: Water Quality Basin

Inflow Area = 1.839 ac, 69.52% Impervious, Inflow Depth > 5.32" for 100-yr event
 Inflow = 9.97 cfs @ 12.02 hrs, Volume= 0.816 af
 Outflow = 6.64 cfs @ 12.15 hrs, Volume= 0.750 af, Atten= 33%, Lag= 7.8 min
 Primary = 6.64 cfs @ 12.15 hrs, Volume= 0.750 af
 Routed to nonexistent node 30

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 32.80' Surf.Area= 3,942 sf Storage= 2,955 cf
 Peak Elev= 34.74' @ 12.15 hrs Surf.Area= 5,178 sf Storage= 11,800 cf (8,846 cf above start)

Plug-Flow detention time= 150.1 min calculated for 0.682 af (84% of inflow)
 Center-of-Mass det. time= 51.2 min (846.4 - 795.3)

Volume	Invert	Avail.Storage	Storage Description			
#1	32.00'	13,158 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
32.00	3,450	299.0	0	0	3,450	
33.00	4,070	311.0	3,756	3,756	4,108	
34.00	4,700	322.0	4,381	8,137	4,746	
35.00	5,350	335.0	5,021	13,158	5,501	

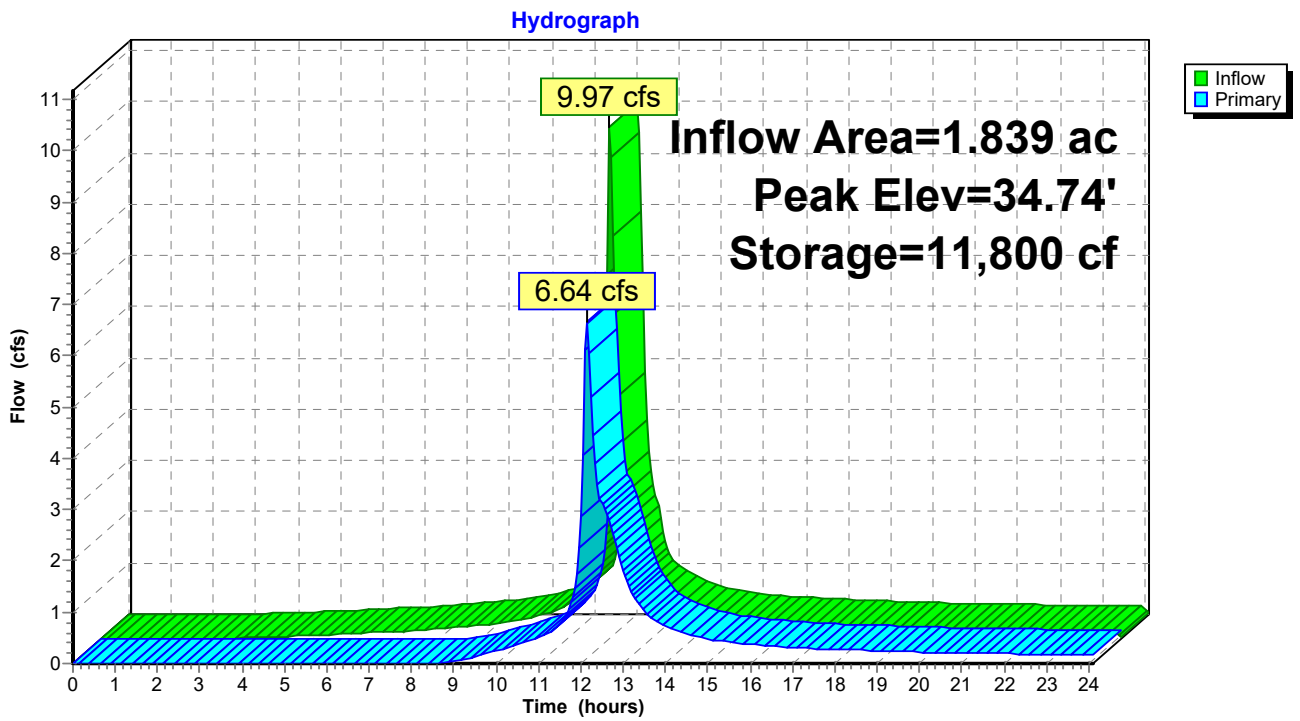
Device	Routing	Invert	Outlet Devices											
#1	Primary	34.50'	10.0' long + 0.5 ' SideZ x 3.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68 2.72 2.81 2.92 2.97 3.07 3.32											
#2	Primary	33.30'	12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads											

Primary OutFlow Max=6.62 cfs @ 12.15 hrs HW=34.74' (Free Discharge)

1=**Broad-Crested Rectangular Weir** (Weir Controls 2.95 cfs @ 1.21 fps)

2=**Orifice/Grate** (Orifice Controls 3.67 cfs @ 4.67 fps)

Pond 20S: Water Quality Basin



Summary for Pond 21SA: Water Quality Basin

Inflow Area = 0.401 ac, 64.15% Impervious, Inflow Depth > 6.22" for 100-yr event
 Inflow = 3.63 cfs @ 11.95 hrs, Volume= 0.208 af
 Outflow = 3.59 cfs @ 11.96 hrs, Volume= 0.208 af, Atten= 1%, Lag= 0.3 min
 Primary = 3.59 cfs @ 11.96 hrs, Volume= 0.208 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Starting Elev= 37.40' Surf.Area= 1,493 sf Storage= 1,899 cf
 Peak Elev= 37.46' @ 11.96 hrs Surf.Area= 1,528 sf Storage= 1,988 cf (89 cf above start)

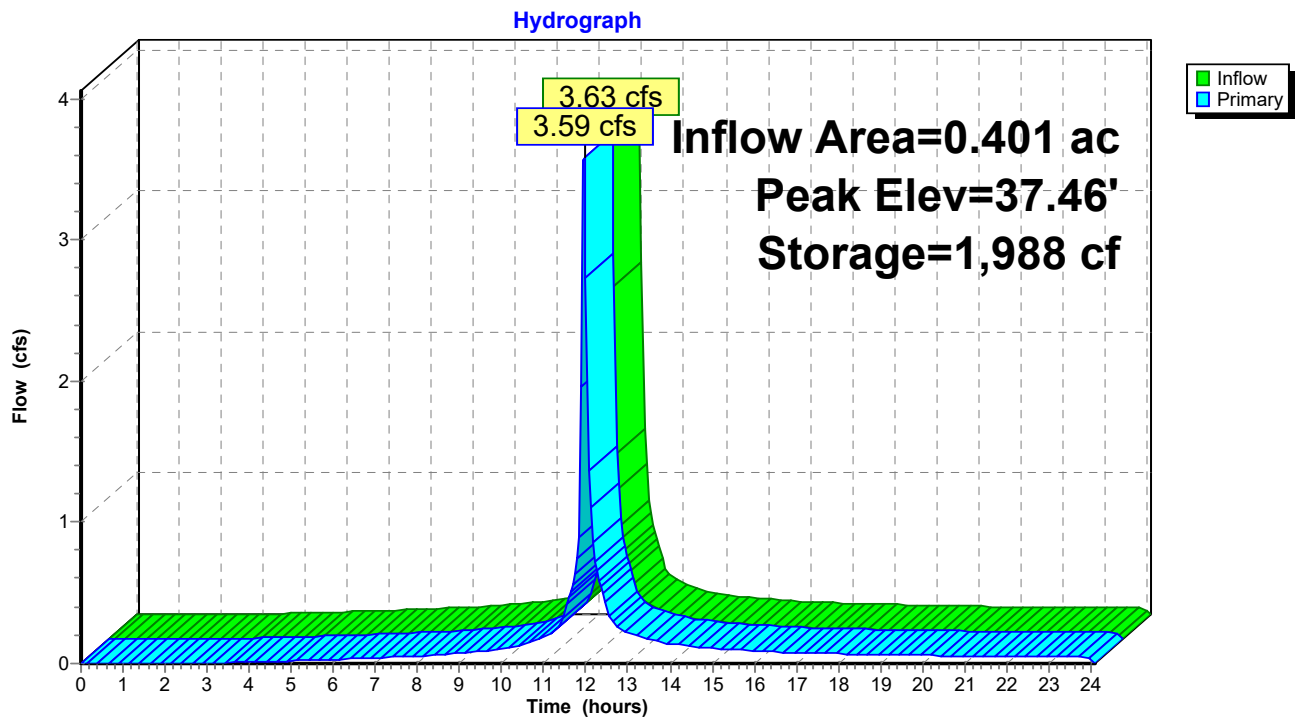
Plug-Flow detention time= 157.3 min calculated for 0.164 af (79% of inflow)
 Center-of-Mass det. time= 0.6 min (796.8 - 796.2)

Volume	Invert	Avail.Storage	Storage Description			
#1	35.00'	2,903 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
35.00	166	238.0	0	0	166	
36.00	681	264.0	394	394	1,235	
37.00	1,259	291.0	955	1,350	2,459	
37.50	1,554	298.0	702	2,052	2,819	
38.00	1,856	304.0	851	2,903	3,143	

Device	Routing	Invert	Outlet Devices
#1	Primary	37.40'	2.4" x 4.0" Horiz. Orifice/Grate X 8.00 columns X 9 rows C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=3.39 cfs @ 11.96 hrs HW=37.46' (Free Discharge)
 ↑1=Orifice/Grate (Weir Controls 3.39 cfs @ 0.78 fps)

Pond 21SA: Water Quality Basin



Summary for Pond 22SB: Underground 22

Inflow Area = 0.370 ac, 78.90% Impervious, Inflow Depth > 6.81" for 100-yr event
 Inflow = 2.94 cfs @ 12.04 hrs, Volume= 0.210 af
 Outflow = 1.68 cfs @ 12.15 hrs, Volume= 0.193 af, Atten= 43%, Lag= 6.6 min
 Discarded = 0.13 cfs @ 12.15 hrs, Volume= 0.149 af
 Primary = 1.55 cfs @ 12.15 hrs, Volume= 0.044 af
 Routed to Pond 20S : Water Quality Basin

Routing by Stor-Ind method, Time Span= 0.00-24.10 hrs, dt= 0.05 hrs
 Peak Elev= 38.67' @ 12.15 hrs Surf.Area= 0.072 ac Storage= 0.057 af

Plug-Flow detention time= 145.2 min calculated for 0.192 af (91% of inflow)
 Center-of-Mass det. time= 98.0 min (881.1 - 783.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	37.00'	0.034 af	27.00'W x 116.39'L x 2.00'H Field A 0.144 af Overall - 0.030 af Embedded = 0.114 af x 30.0% Voids
#2A	37.50'	0.030 af	ADS_StormTech SC-160LP +Cap x 192 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 192 Chambers in 12 Rows
		0.064 af	Total Available Storage

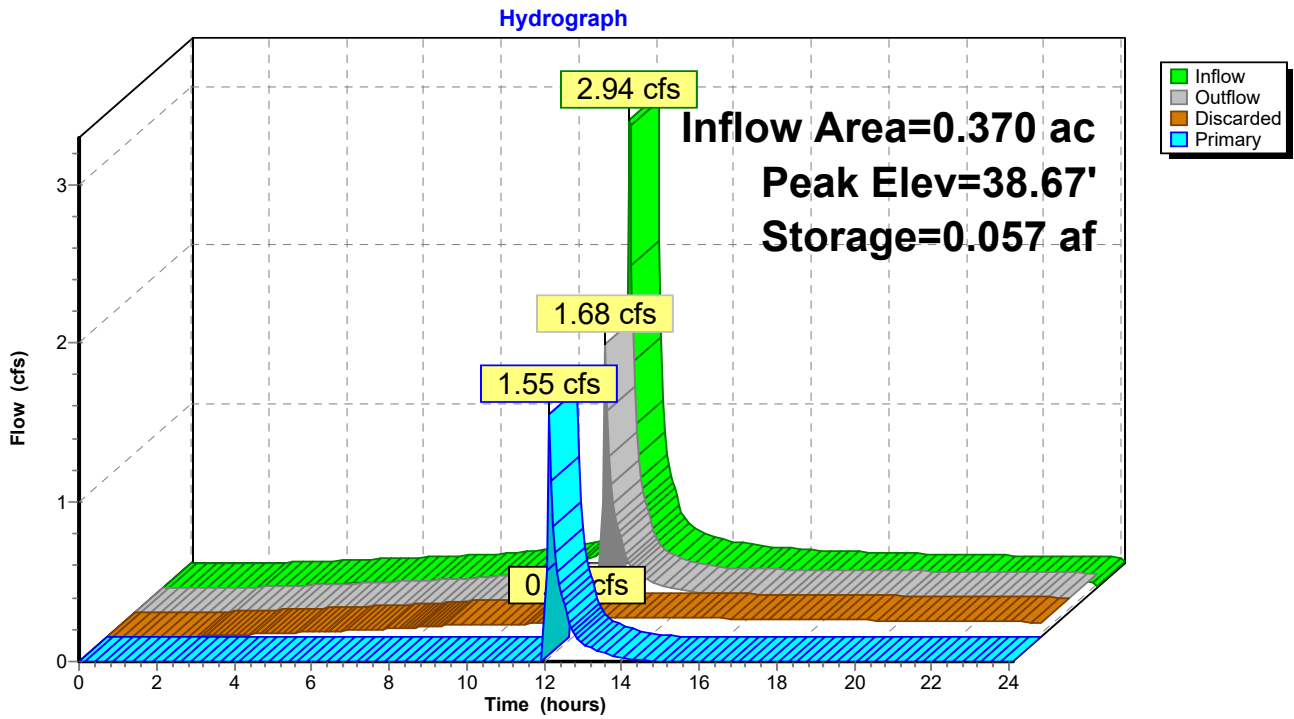
Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	37.00'	1.000 in/hr Exfiltration over Wetted area Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.40'	4.0' long + 1.0 ' SideZ x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.13 cfs @ 12.15 hrs HW=38.66' (Free Discharge)
 ↑1=Exfiltration (Controls 0.13 cfs)

Primary OutFlow Max=1.55 cfs @ 12.15 hrs HW=38.66' (Free Discharge)
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 1.55 cfs @ 1.37 fps)

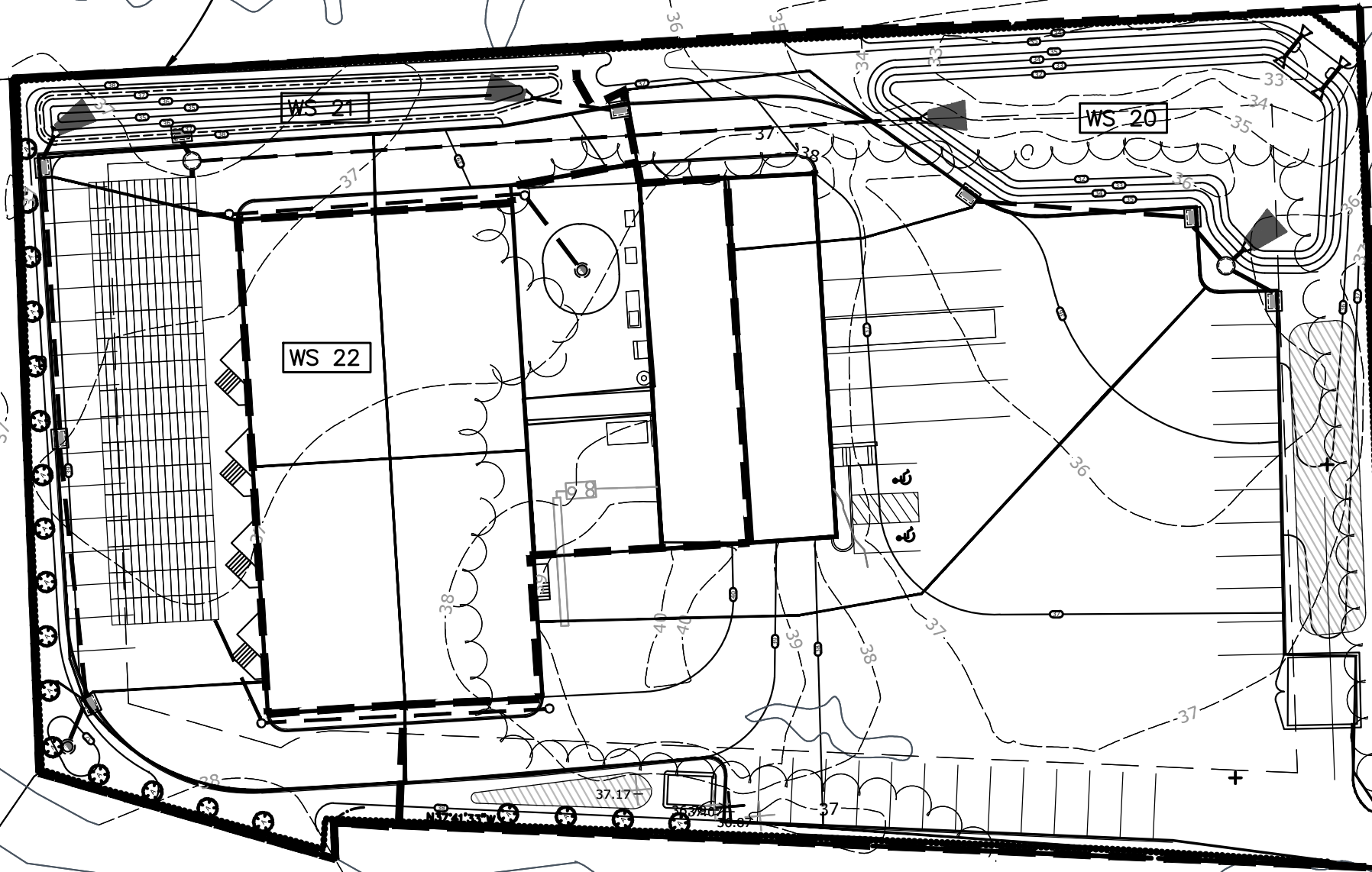
Pond 22SB: Underground 22



Appendix C
Pipe Capacity Calculations

DATE	REVISION	CK.
12/12/22	TOWN COMMENTS	
01/06/23	TOWN COMMENTS	
02/06/23	TOWN COMMENTS	

PROPOSED WATERSHED BOUNDARY




WS 22

GRAPHIC SCALE



(IN FEET)
1 inch = 40 ft.



DOANE ENGINEERING
CIVIL ENGINEERING AND LAND SURVEYING
P.O. BOX 113 CENTERBROOK, CONNECTICUT 06409
TEL: (860)767-0138, FAX: (860)767-9104

CATCH BASIN CATCHMENT AREAS
PREPARED FOR
PIAGE MANAGEMENT CORP
#49 PLAINS ROAD, ESSEX, CONNECTICUT

SCALE: 1"=40'	DATE: 11/29/22	SHEET NO.: 1 OF 1	IDENT. NO.:
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FILE: DRAWINGS\TOWNS\ESSEX\PLAINS ROAD\PIAGENTM-BOB\Hydro 03 Areas.dwg

Rational Method Individual Basin Calculations

Catch Basin and Area Drain Runoff Coefficients

Basin Name	Impervious Area C=0.9 (sf)	Grass Area C=0.3 (sf)	Wooded Area C=0.2 (sf)	Total Area (sf)	Total Area (ac)	Weighted C	Tc to Inlet (min)
CCB 3	9,950	0	0	9,950	0.23	0.90	5.0
CCB 4	2,610	250	0	2,860	0.07	0.85	5.0
CLCB 5	19,000	0	0	19,000	0.44	0.90	5.0
CCB 9	1,176	175	0	1,351	0.03	0.82	5.0
CCB 15	1,275	175	0	1,450	0.03	0.83	5.0
CCB 16	7,088	0	0	7,088	0.16	0.90	5.0
CCB 17	1,668	175	0	1,843	0.04	0.84	5.0
YD 17A	0	1500	0	1,500	0.03	0.30	5.0
YD 11A	2,660	3320	0	5,980	0.14	0.57	5.0

Roof Drainage Pipe Calculations

Q = C x I x A, Where:

C = Runoff Coefficient

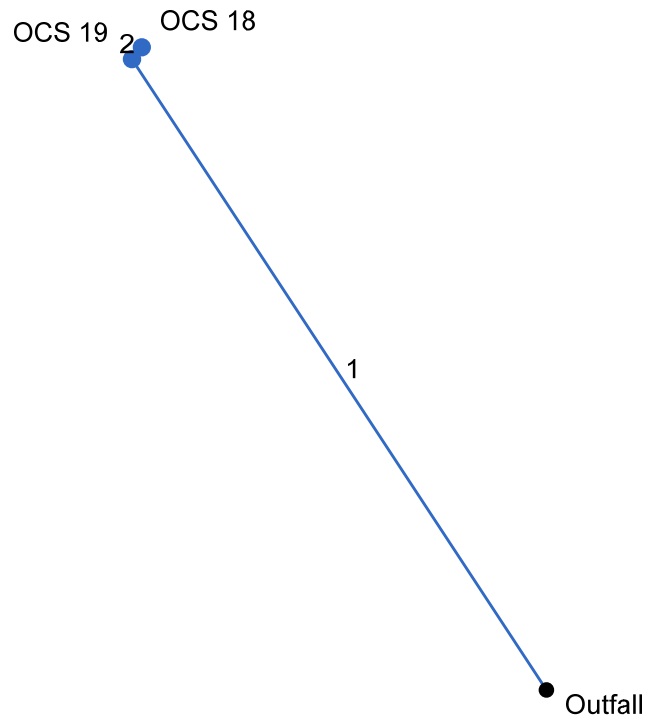
I = Rainfall Intensity (in/hr)

A = Area (acres)

Q = Flow (cfs)

	MH 9	MH 10	MH 11	MH 12
C	0.90	0.90	0.90	0.90
I	8.83	8.83	8.83	8.83
A	0.06	0.06	0.06	0.06
Q	0.48	0.48	0.48	0.48

System 20 OCS 100 YR



Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data							Line ID	
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)		Inlet/ Rim El (ft)
1	End	196.000	-123.199	MH	1.55	0.00	0.00	0.0	32.50	0.77	34.00	15	Cir	0.013	0.96	39.90	OCS 19-FES 20
2	1	4.000	72.809	MH	3.59	0.00	0.00	0.0	34.00	5.00	34.20	15	Cir	0.013	1.00	37.40	OCS 18-OCS 19

System 20 OCS 100 YR

Number of lines: 2

Date: 2/9/2023

Storm Sewer Tabulation

Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr (ac)	Total (ac)		Incr	Total	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	
1	End	196.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	5.14	5.65	4.19	15	0.77	32.50	34.00	34.74	35.98	33.75	39.90	OCS 19-FES 20
2	1	4.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	3.59	14.44	2.93	15	5.00	34.00	34.20	36.24	36.26	39.90	37.40	OCS 18-OCS 19

System 20 OCS 100 YR

Number of lines: 2

Run Date: 2/9/2023

NOTES: Intensity = 50.44 / (Inlet time + 3.60) ^ 0.70; Return period = Yrs. 100 ; c = cir e = ellip b = box

Hydraulic Grade Line Computations

Line	Size (in)	Q (cfs)	Downstream								Len (ft)	Upstream								Check		JL coeff (K)	Minor loss (ft)
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
1	15	5.14	32.50	34.74	1.25	1.23	4.19	0.27	35.01	0.634	196.00	34.00	35.98	1.25	1.23	4.19	0.27	36.25	0.634	0.634	1.242	0.96	0.26
2	15	3.59	34.00	36.24	1.25	1.23	2.93	0.13	36.38	0.309	4.000	34.20	36.26	1.25	1.23	2.93	0.13	36.39	0.309	0.309	0.012	1.00	0.13

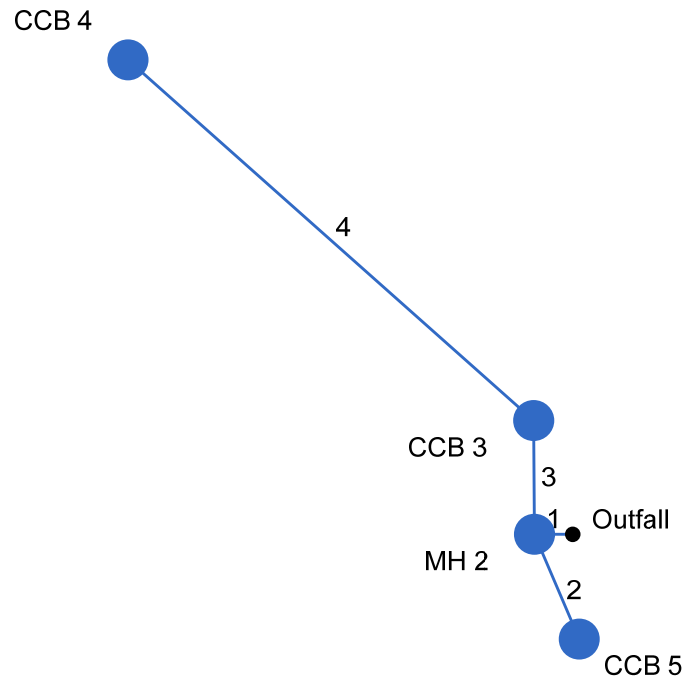
System 20 OCS 100 YR

Number of lines: 2

Run Date: 2/9/2023

; c = cir e = ellip b = box

System 20 25 YR



Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data							Line ID	
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)		Inlet/ Rim El (ft)
1	End	4.000	180.000	MH	0.00	0.00	0.00	0.0	32.30	5.00	32.50	15	Cir	0.013	1.00	35.80	MH2-FES1
2	1	12.000	-112.962	Comb	0.00	0.44	0.90	5.0	32.50	1.67	32.70	15	Cir	0.013	1.00	35.40	CCB 5-MH 2
3	1	12.000	89.593	Comb	0.00	0.23	0.90	5.0	32.50	1.67	32.70	15	Cir	0.013	1.17	35.80	CCB 3-MH 2
4	3	57.000	-47.735	Comb	0.00	0.07	0.85	5.0	32.70	1.93	33.80	15	Cir	0.013	1.00	36.10	CCB 4-CCB 3
System 20 25 YR												Number of lines: 4				Date: 2/9/2023	

Storm Sewer Tabulation

Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID	
Line	To Line		Incr (ac)	Total (ac)		Incr	Total	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)		
1	End	4.000	0.00	0.74	0.00	0.00	0.66	0.0	7.2	7.5	4.98	14.44	4.06	15	5.00	32.30	32.50	34.49	34.51	33.25	35.80	MH2-FES1	
2	1	12.000	0.44	0.44	0.90	0.40	0.40	5.0	5.0	8.8	3.49	8.34	2.84	15	1.67	32.50	32.70	34.77	34.80	35.80	35.40	CCB 5-MH 2	
3	1	12.000	0.23	0.30	0.90	0.21	0.27	5.0	7.1	7.6	2.02	8.34	1.64	15	1.67	32.50	32.70	34.77	34.78	35.80	35.80	CCB 3-MH 2	
4	3	57.000	0.07	0.07	0.85	0.06	0.06	5.0	5.0	8.8	0.52	8.97	0.45	15	1.93	32.70	33.80	34.83	34.83	35.80	36.10	CCB 4-CCB 3	
System 20 25 YR																Number of lines: 4				Run Date: 2/9/2023			
NOTES: Intensity = $40.94 / (\text{Inlet time} + 3.80)^{0.71}$; Return period = Yrs. 25 ; c = cir e = ellip b = box																							

Inlet Report

Line No	Inlet ID	Q = CIA (cfs)	Q carry (cfs)	Q capt (cfs)	Q Byp (cfs)	Junc Type	Curb Inlet		Grate Inlet			Gutter						Inlet			Byp Line No		
							Ht (in)	L (ft)	Area (sqft)	L (ft)	W (ft)	So (ft/ft)	W (ft)	Sw (ft/ft)	Sx (ft/ft)	n	Depth (ft)	Spread (ft)	Depth (ft)	Spread (ft)		Depr (in)	
1	MH 2	0.00	0.00	0.00	0.00	MH	0.0	0.00	0.00	0.00	0.00	Sag	0.00	0.000	0.000	0.000	0.00	0.00	0.00	0.00	0.00	0.0	Off
2	CCB 5	3.49	0.00	3.49	0.00	Comb	4.0	2.73	3.12	2.31	1.35	Sag	2.00	0.010	0.010	0.000	0.37	36.70	0.37	36.70	0.0	Off	
3	CCB 3	1.82	0.26	2.09	0.00	Comb	4.0	2.73	3.12	2.31	1.35	Sag	2.00	0.010	0.010	0.000	0.26	26.32	0.26	26.32	0.0	Off	
4	CCB 4	0.52	0.00	0.26	0.26	Comb	4.0	2.73	0.00	2.31	1.35	0.010	2.00	0.010	0.010	0.013	0.08	8.07	0.06	6.24	0.0	3	

System 20 25 YR Number of lines: 4 Run Date: 2/9/2023

NOTES: Inlet N-Values = 0.016; Intensity = 40.94 / (Inlet time + 3.80) ^ 0.71; Return period = 25 Yrs. ; * Indicates Known Q added. All curb inlets are Horiz throat.

Hydraulic Grade Line Computations

Line	Size (in)	Q (cfs)	Downstream								Len (ft)	Upstream								Check		JL coeff (K)	Minor loss (ft)
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
1	15	4.98	32.30	34.49	1.25	1.23	4.06	0.26	34.75	0.595	4.000	32.50	34.51	1.25	1.23	4.06	0.26	34.77	0.594	0.594	0.024	1.00	0.26
2	15	3.49	32.50	34.77	1.25	1.23	2.84	0.13	34.90	0.291	12.000	32.70	34.80	1.25	1.23	2.84	0.13	34.93	0.291	0.291	0.035	1.00	0.13
3	15	2.02	32.50	34.77	1.25	1.23	1.65	0.04	34.81	0.098	12.000	32.70	34.78	1.25	1.23	1.64	0.04	34.82	0.098	0.098	0.012	1.17	0.05
4	15	0.52	32.70	34.83	1.25	1.23	0.43	0.00	34.83	0.007	57.000	33.80	34.83	1.03	1.09	0.48	0.00	34.84	0.006	0.007	0.004	1.00	0.00

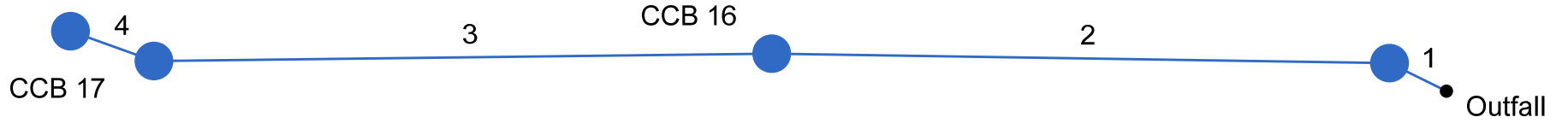
System 20 25 YR

Number of lines: 4

Run Date: 2/9/2023

; c = cir e = ellip b = box

System 21A 25 YR



Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data								Line ID
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
1	End	7.000	-153.538	MH	0.00	0.03	0.83	5.0	35.10	1.43	35.20	12	Cir	0.013	0.49	38.40	CCB 15-FES 14
2	1	68.000	-25.581	Comb	0.00	0.16	0.90	5.0	35.20	0.59	35.60	12	Cir	0.013	0.50	38.40	CCB 16-CCB 15
3	2	68.000	-1.565	Comb	0.00	0.04	0.84	5.0	35.60	0.59	36.00	12	Cir	0.013	0.61	38.40	CCB 17-CCB 16
4	3	9.768	20.530	DrGrt	0.00	0.03	0.30	5.0	36.00	1.02	36.10	6	Cir	0.011	1.00	36.80	YD 17A-CCB 17
System 21A 25 YR												Number of lines: 4				Date: 2/9/2023	

Storm Sewer Tabulation

Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID	
Line	To Line		Incr (ac)	Total (ac)		Incr	Total	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)		
1	End	7.000	0.03	0.26	0.83	0.02	0.21	5.0	8.5	6.9	1.47	4.26	1.87	12	1.43	35.10	35.20	37.45	37.46	36.10	38.40	CCB 15-FES 14	
2	1	68.000	0.16	0.23	0.90	0.14	0.19	5.0	7.9	7.2	1.35	2.73	1.71	12	0.59	35.20	35.60	37.49	37.59	38.40	38.40	CCB 16-CCB 15	
3	2	68.000	0.04	0.07	0.84	0.03	0.04	5.0	5.4	8.5	0.36	2.73	0.46	12	0.59	35.60	36.00	37.61	37.62	38.40	38.40	CCB 17-CCB 16	
4	3	9.768	0.03	0.03	0.30	0.01	0.01	5.0	5.0	8.8	0.08	0.67	0.40	6	1.02	36.00	36.10	37.62	37.62	38.40	36.80	YD 17A-CCB 17	
System 21A 25 YR																Number of lines: 4				Run Date: 2/9/2023			
NOTES: Intensity = $40.94 / (\text{Inlet time} + 3.80)^{0.71}$; Return period = Yrs. 25 ; c = cir e = ellip b = box																							

Inlet Report

Line No	Inlet ID	Q = CIA (cfs)	Q carry (cfs)	Q capt (cfs)	Q Byp (cfs)	Junc Type	Curb Inlet		Grate Inlet			Gutter						Inlet			Byp Line No		
							Ht (in)	L (ft)	Area (sqft)	L (ft)	W (ft)	So (ft/ft)	W (ft)	Sw (ft/ft)	Sx (ft/ft)	n	Depth (ft)	Spread (ft)	Depth (ft)	Spread (ft)		Depr (in)	
1		0.22	0.00	0.00	0.22	MH	0.0	0.00	0.00	0.00	0.00	Sag	0.00	0.000	0.000	0.000	0.00	0.00	0.00	0.00	0.00	0.0	Off
2	CCB 16	1.27	0.00	1.27	0.00	Comb	4.0	2.73	3.12	2.31	1.35	Sag	2.00	0.028	0.028	0.000	0.21	7.43	0.21	7.43	0.0	Off	
3	CCB 17	0.30	0.00	0.30	0.00	Comb	4.0	2.73	3.12	2.31	1.35	Sag	2.00	0.028	0.028	0.000	0.09	3.39	0.09	3.39	0.0	Off	
4		0.08	0.00	0.08	0.00	DrGrt	4.0	4.00	2.00	2.00	2.00	Sag	2.00	0.050	0.020	0.013	0.02	4.21	0.02	4.21	0.0	3	

System 21A 25 YR Number of lines: 4 Run Date: 2/9/2023

NOTES: Inlet N-Values = 0.016; Intensity = 40.94 / (Inlet time + 3.80) ^ 0.71; Return period = 25 Yrs. ; * Indicates Known Q added. All curb inlets are Horiz throat.

Hydraulic Grade Line Computations

Line	Size (in)	Q (cfs)	Downstream								Len (ft)	Upstream								Check		JL coeff (K)	Minor loss (ft)
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
1	12	1.47	35.10	37.45	1.00	0.79	1.87	0.05	37.50	0.170	7.000	35.20	37.46	1.00	0.79	1.87	0.05	37.52	0.170	0.170	0.012	0.49	0.03
2	12	1.35	35.20	37.49	1.00	0.79	1.72	0.05	37.53	0.143	68.000	35.60	37.59	1.00	0.79	1.71	0.05	37.63	0.143	0.143	0.097	0.50	0.02
3	12	0.36	35.60	37.61	1.00	0.79	0.46	0.00	37.61	0.010	68.000	36.00	37.62	1.00	0.79	0.46	0.00	37.62	0.010	0.010	0.007	0.61	0.00
4	6	0.08	36.00	37.62	0.50	0.20	0.40	0.00	37.62	0.014	9.768	36.10	37.62	0.50	0.20	0.40	0.00	37.62	0.014	0.014	0.001	1.00	0.00

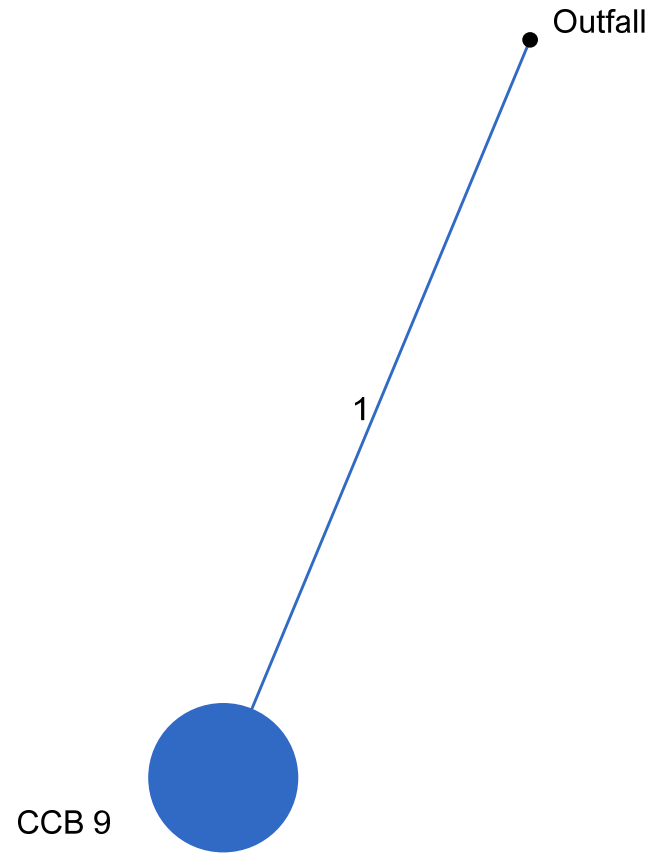
System 21A 25 YR

Number of lines: 4

Run Date: 2/9/2023

; c = cir e = ellip b = box

System 21B 25 YR



Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data								Line ID
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
1	End	22.000	112.279	Comb	0.00	0.90	0.44	5.0	35.20	1.82	35.60	12	Cir	0.013	1.00	37.80	CCB 9-FES 8

System 21B 25 YR

Number of lines: 1

Date: 2/6/2023

Storm Sewer Tabulation

Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID	
Line	To Line		Incr (ac)	Total (ac)		Incr	Total	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)		
1	End	22.000	0.90	0.90	0.44	0.40	0.40	5.0	5.0	8.8	3.49	4.80	4.44	12	1.82	35.20	35.60	37.45	37.66	36.20	37.80	CCB 9-FES 8	
System 21B 25 YR																Number of lines: 1				Run Date: 2/6/2023			
NOTES: Intensity = $40.94 / (\text{Inlet time} + 3.80)^{0.71}$; Return period = Yrs. 25 ; c = cir e = ellip b = box																							

Inlet Report

Line No	Inlet ID	Q = CIA (cfs)	Q carry (cfs)	Q capt (cfs)	Q Byp (cfs)	Junc Type	Curb Inlet		Grate Inlet			Gutter						Inlet			Byp Line No	
							Ht (in)	L (ft)	Area (sqft)	L (ft)	W (ft)	So (ft/ft)	W (ft)	Sw (ft/ft)	Sx (ft/ft)	n	Depth (ft)	Spread (ft)	Depth (ft)	Spread (ft)		Depr (in)
1	CCB 9	3.49	0.00	1.38	2.10	Comb	4.0	2.73	0.00	1.35	2.31	0.040	2.00	0.010	0.010	0.013	0.13	12.67	0.10	10.49	0.0	Off

System 21B 25 YR Number of lines: 1 Run Date: 2/6/2023

NOTES: Inlet N-Values = 0.016; Intensity = 40.94 / (Inlet time + 3.80) ^ 0.71; Return period = 25 Yrs. ; * Indicates Known Q added. All curb inlets are Horiz throat.

Hydraulic Grade Line Computations

Line	Size (in)	Q (cfs)	Downstream								Len (ft)	Upstream								Check		JL coeff (K)	Minor loss (ft)
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
1	12	3.49	35.20	37.45	1.00	0.79	4.44	0.31	37.76	0.958	22.000	35.60	37.66	1.00	0.79	4.44	0.31	37.97	0.958	0.958	0.211	1.00	0.31

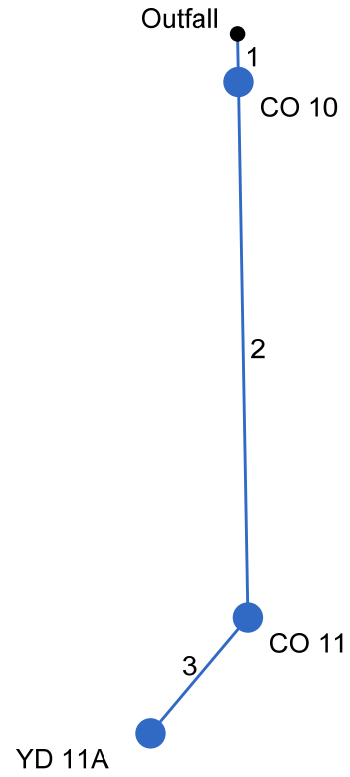
System 21B 25 YR

Number of lines: 1

Run Date: 2/6/2023

; c = cir e = ellip b = box

System 22A 25 YR



Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data							Line ID	
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)		Inlet/ Rim El (ft)
1	End	7.000	88.960	MH	0.48	0.00	0.00	0.0	37.50	1.43	37.60	8	Cir	0.010	0.15	40.00	CO 10-UG 22SB
2	1	78.000	0.034	MH	0.48	0.00	0.00	0.0	37.60	0.51	38.00	8	Cir	0.010	0.70	40.00	CO 11-CO 10
3	2	22.000	40.954	Grate	0.00	0.14	0.57	5.0	38.00	0.91	38.20	8	Cir	0.010	1.00	39.50	YD 11A-CO 11
System 22A 25 YR												Number of lines: 3				Date: 2/9/2023	

Storm Sewer Tabulation

Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr (ac)	Total (ac)		Incr	Total	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	
1	End	7.000	0.00	0.14	0.00	0.00	0.08	0.0	5.6	8.4	1.63	1.88	4.68	8	1.43	37.50	37.60	38.50	38.58	38.50	40.00	CO 10-UG 22SB
2	1	78.000	0.00	0.14	0.00	0.00	0.08	0.0	5.2	8.7	1.17	1.12	3.36	8	0.51	37.60	38.00	38.63	39.06	40.00	40.00	CO 11-CO 10
3	2	22.000	0.14	0.14	0.57	0.08	0.08	5.0	5.0	8.8	0.70	1.50	2.01	8	0.91	38.00	38.20	39.18	39.23	40.00	39.50	YD 11A-CO 11

System 22A 25 YR

Number of lines: 3

Run Date: 2/9/2023

NOTES: Intensity = $40.94 / (\text{Inlet time} + 3.80)^{0.71}$; Return period = Yrs. 25 ; c = cir e = ellip b = box

Inlet Report

Line No	Inlet ID	Q = CIA (cfs)	Q carry (cfs)	Q capt (cfs)	Q Byp (cfs)	Junc Type	Curb Inlet		Grate Inlet			Gutter						Inlet			Byp Line No	
							Ht (in)	L (ft)	Area (sqft)	L (ft)	W (ft)	So (ft/ft)	W (ft)	Sw (ft/ft)	Sx (ft/ft)	n	Depth (ft)	Spread (ft)	Depth (ft)	Spread (ft)		Depr (in)
1	CO 10	0.48*	0.48	0.00	0.96	MH	0.0	0.00	0.00	0.00	0.00	Sag	2.00	0.050	0.020	0.013	0.00	0.00	0.00	0.00	0.00	Off
2	CO 11	0.48*	0.00	0.00	0.48	MH	0.0	0.00	0.00	0.00	0.00	Sag	2.00	0.050	0.020	0.013	0.00	0.00	0.00	0.00	0.00	1
3	YD 11A	0.70	0.00	0.70	0.00	Grate	0.0	0.00	2.00	2.00	2.00	Sag	2.00	0.050	0.020	0.013	0.16	5.25	0.16	5.25	0.0	2

System 22A 25 YR

Number of lines: 3

Run Date: 2/9/2023

NOTES: Inlet N-Values = 0.016; Intensity = 40.94 / (Inlet time + 3.80) ^ 0.71; Return period = 25 Yrs. ; * Indicates Known Q added. All curb inlets are Horiz throat.

Hydraulic Grade Line Computations

Line	Size (in)	Q (cfs)	Downstream								Len (ft)	Upstream								Check		JL coeff (K)	Minor loss (ft)
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
1	8	1.63	37.50	38.50	0.67	0.35	4.68	0.34	38.84	1.081	7.000	37.60	38.58	0.67	0.35	4.68	0.34	38.92	1.080	1.081	0.076	0.15	0.05
2	8	1.17	37.60	38.63	0.67	0.35	3.36	0.18	38.80	0.558	78.000	38.00	39.06	0.67	0.35	3.36	0.18	39.24	0.558	0.558	0.435	0.70	0.12
3	8	0.70	38.00	39.18	0.67	0.35	2.01	0.06	39.25	0.200	22.000	38.20	39.23	0.67	0.35	2.01	0.06	39.29	0.200	0.200	0.044	1.00	0.06

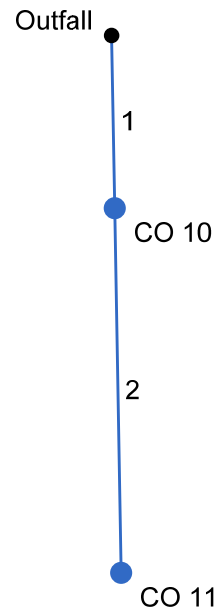
System 22A 25 YR

Number of lines: 3

Run Date: 2/9/2023

; c = cir e = ellip b = box

System 22B 25 YR



Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data							Line ID	
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)		Inlet/ Rim El (ft)
1	End	36.000	88.960	MH	0.48	0.00	0.00	0.0	37.50	0.56	37.70	8	Cir	0.010	0.15	40.00	CO 12-UG 22SB
2	1	76.000	0.034	MH	0.48	0.00	0.00	0.0	37.70	0.66	38.20	8	Cir	0.010	1.00	40.00	CO 13-CO 12

System 22B 25 YR

Number of lines: 2

Date: 2/9/2023

Storm Sewer Tabulation

Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr (ac)	Total (ac)		Incr	Total	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	
1	End	36.000	0.00	0.00	0.00	0.00	0.00	0.0	0.8	0.0	0.96	1.17	2.75	8	0.56	37.50	37.70	38.50	38.63	38.50	40.00	CO 12-UG 22SB
2	1	76.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	0.48	1.27	1.52	8	0.66	37.70	38.20	38.65	38.71	40.00	40.00	CO 13-CO 12

System 22B 25 YR

Number of lines: 2

Run Date: 2/9/2023

NOTES: Intensity = $40.94 / (\text{Inlet time} + 3.80)^{0.71}$; Return period = Yrs. 25 ; c = cir e = ellip b = box

Hydraulic Grade Line Computations

Line	Size (in)	Q (cfs)	Downstream								Len (ft)	Upstream								Check		JL coeff (K)	Minor loss (ft)
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
1	8	0.96	37.50	38.50	0.67	0.35	2.75	0.12	38.62	0.374	36.000	37.70	38.63	0.67	0.35	2.75	0.12	38.75	0.374	0.374	0.135	0.15	0.02
2	8	0.48	37.70	38.65	0.67	0.35	1.38	0.03	38.68	0.094	76.000	38.20	38.71	0.51	0.29	1.66	0.04	38.76	0.105	0.099	0.076	1.00	0.04

System 22B 25 YR

Number of lines: 2

Run Date: 2/9/2023

; c = cir e = ellip b = box

Outlet Protection Calculations

Outlet I.D. **FES 1**

*Based on Connecticut DOT Drainage Manual, Section 11.13

Description:

FES 1

Design Criteria (25-yr Storm Event):

Q (cfs) = 4.98	R _p (ft)= 1.25
D (in) = 15	S _p (ft) = 1.25
V (fps) = 4.06	T _w (ft)= 2.19

Q= Flow rate at discharge point in cubic feet per second (cfs)

D= Outlet pipe diameter (in)

V= Flow velocity at discharge point (ft/s)

R_p= Maximum inside pipe rise (ft)

S_p= inside diameters for circular sections of maximum inside pipe span for non-circular sections (ft)

T_w= Tailwater depth (ft)

Based on **Table 11-13.1** use Type 'B' ---> TW ≥ 0.5 R_p

Rip Rap Stone Size:

<u>Velocity</u>	<u>Rip Rap Specification</u>	<u>D₅₀ Stone Size</u>
0-8 fps	Modified	5 inches

Preformed Scour Hole Dimensions:

F(ft)=0.5(R _p)	=	n/a
C(ft)=3.0(S _p)+6.0(F)	=	n/a
B(ft)=2.0(S _p)+6.0(F)	=	n/a

Rip Rap Splash Pad Dimensions:

L _a	=	10	ft
W1 = 3.0(S _p) min.	=	4	ft
W2 = 3.0(S _p)+0.4(L _a) min.	=	8	ft
d (Depth of Stone)	=	12	inches

Outlet Protection Calculations

Outlet I.D. **FES 8**

*Based on Connecticut DOT Drainage Manual, Section 11.13

Description:

FES 8

Design Criteria (25-yr Storm Event):

Q (cfs) = 3.49 R_p (ft) = 1
D (in) = 12 S_p (ft) = 1
V (fps) = 4.44 T_w (ft) = 2.25

Q = Flow rate at discharge point in cubic feet per second (cfs)

D = Outlet pipe diameter (in)

V = Flow velocity at discharge point (ft/s)

R_p = Maximum inside pipe rise (ft)

S_p = inside diameter for circular sections of maximum inside pipe span for non-circular sections (ft)

T_w = Tailwater depth (ft)

Based on **Table 11-13.1** use Type 'B' ---> TW ≥ 0.5 R_p

Rip Rap Stone Size:

<u>Velocity</u>	<u>Rip Rap Specification</u>	<u>D₅₀ Stone Size</u>
0-8 fps	Modified	5 inches

Preformed Scour Hole Dimensions:

F(ft) = 0.5(R_p) = n/a
C(ft) = 3.0(S_p) + 6.0(F) = n/a
B(ft) = 2.0(S_p) + 6.0(F) = n/a

Rip Rap Splash Pad Dimensions:

L_a = 10 ft
W1 = 3.0(S_p) min. = 3 ft
W2 = 3.0(S_p) + 0.4(L_a) min. = 7 ft
d (Depth of Stone) = 12 inches

Outlet Protection Calculations

Outlet I.D. **FES 14**

*Based on Connecticut DOT Drainage Manual, Section 11.13

Description:

FES 14

Design Criteria (25-yr Storm Event):

Q (cfs) = 1.47 R_p (ft) = 1
D (in) = 12 S_p (ft) = 1
V (fps) = 1.87 T_w (ft) = 2.35

Q = Flow rate at discharge point in cubic feet per second (cfs)

D = Outlet pipe diameter (in)

V = Flow velocity at discharge point (ft/s)

R_p = Maximum inside pipe rise (ft)

S_p = inside diameter for circular sections of maximum inside pipe span for non-circular sections (ft)

T_w = Tailwater depth (ft)

Based on **Table 11-13.1** use Type 'B' ---> TW ≥ 0.5 R_p

Rip Rap Stone Size:

<u>Velocity</u>	<u>Rip Rap Specification</u>	<u>D₅₀ Stone Size</u>
0-8 fps	Modified	5 inches

Preformed Scour Hole Dimensions:

F (ft) = 0.5(R_p) = n/a
C (ft) = 3.0(S_p) + 6.0(F) = n/a
B (ft) = 2.0(S_p) + 6.0(F) = n/a

Rip Rap Splash Pad Dimensions:

L_a = 10 ft
W1 = 3.0(S_p) min. = 3 ft
W2 = 3.0(S_p) + 0.4(L_a) min. = 7 ft
d (Depth of Stone) = 12 inches

Outlet Protection Calculations

Outlet I.D. **FES 20**

*Based on Connecticut DOT Drainage Manual, Section 11.13

Description:

FES 20

Design Criteria (100-yr Storm Event):

Q (cfs) = 5.14	R _p (ft)= 1.25
D (in) = 15	S _p (ft) = 1.25
V (fps) = 4.19	T _w (ft)= 2.24

Q= Flow rate at discharge point in cubic feet per second (cfs)

D= Outlet pipe diameter (in)

V= Flow velocity at discharge point (ft/s)

R_p= Maximum inside pipe rise (ft)

S_p= inside diameters for circular sections of maximum inside pipe span for non-circular sections (ft)

T_w= Tailwater depth (ft)

Based on **Table 11-13.1** use Type 'B' ---> TW ≥ 0.5 R_p

Rip Rap Stone Size:

<u>Velocity</u>	<u>Rip Rap Specification</u>	<u>D₅₀ Stone Size</u>
0-8 fps	Modified	5 inches

Preformed Scour Hole Dimensions:

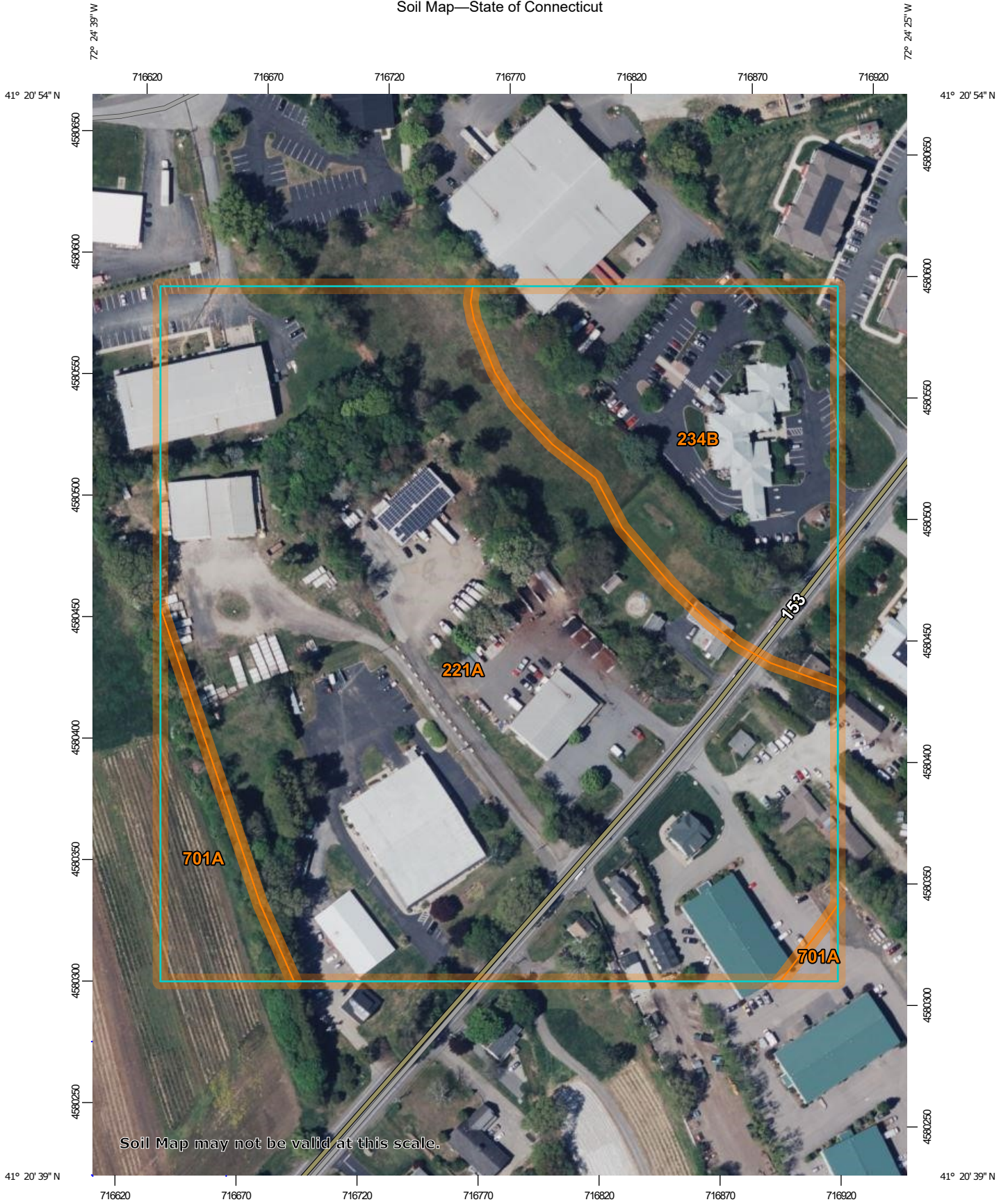
F(ft)=0.5(R _p)	=	n/a
C(ft)=3.0(S _p)+6.0(F)	=	n/a
B(ft)=2.0(S _p)+6.0(F)	=	n/a

Rip Rap Splash Pad Dimensions:

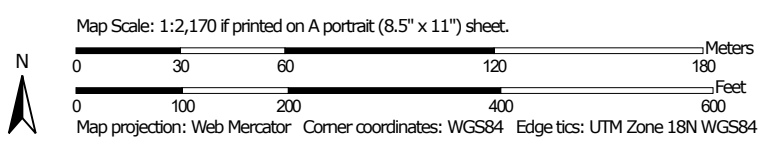
L _a	=	10	ft
W1 = 3.0(S _p) min.	=	4	ft
W2 = 3.0(S _p)+0.4(L _a) min.	=	8	ft
d (Depth of Stone)	=	12	inches

Appendix D
NCRS Soils Information

Soil Map—State of Connecticut




Soil Map may not be valid at this scale.



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: State of Connecticut

Survey Area Data: Version 22, Sep 12, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Data not available.

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
221A	Ninigret-Urban land complex, 0 to 5 percent slopes	14.7	74.3%
234B	Merrimac-Urban land complex, 0 to 8 percent slopes	4.0	20.2%
701A	Ninigret fine sandy loam, 0 to 3 percent slopes	1.1	5.5%
Totals for Area of Interest		19.9	100.0%

Appendix E
NOAA Atlas 14 Precipitation Information



NOAA Atlas 14, Volume 10, Version 3
Location name: Essex, Connecticut, USA*
Latitude: 41.3468°, Longitude: -72.4094°
Elevation: 35.92 ft**
* source: ESRI Maps
** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps & aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.338 (0.259-0.433)	0.406 (0.310-0.520)	0.517 (0.394-0.665)	0.609 (0.462-0.787)	0.736 (0.542-0.984)	0.831 (0.602-1.13)	0.931 (0.656-1.31)	1.04 (0.779-1.48)	1.21 (0.779-1.76)	1.34 (0.846-1.98)
10-min	0.478 (0.366-0.613)	0.575 (0.440-0.737)	0.733 (0.558-0.943)	0.863 (0.655-1.12)	1.04 (0.768-1.39)	1.18 (0.851-1.60)	1.32 (0.930-1.85)	1.48 (0.991-2.10)	1.71 (1.11-2.49)	1.90 (1.20-2.81)
15-min	0.563 (0.431-0.722)	0.676 (0.517-0.867)	0.861 (0.657-1.11)	1.01 (0.770-1.31)	1.23 (0.904-1.64)	1.39 (1.00-1.88)	1.55 (1.09-2.17)	1.74 (1.17-2.47)	2.01 (1.30-2.93)	2.23 (1.41-3.31)
30-min	0.781 (0.599-1.00)	0.938 (0.718-1.20)	1.20 (0.911-1.54)	1.41 (1.07-1.82)	1.70 (1.25-2.27)	1.92 (1.39-2.61)	2.15 (1.52-3.01)	2.41 (1.62-3.43)	2.78 (1.80-4.06)	3.09 (1.95-4.58)
60-min	1.00 (0.766-1.28)	1.20 (0.919-1.54)	1.53 (1.17-1.97)	1.80 (1.37-2.32)	2.17 (1.60-2.91)	2.46 (1.78-3.34)	2.75 (1.94-3.85)	3.08 (2.07-4.38)	3.56 (2.30-5.19)	3.94 (2.50-5.85)
2-hr	1.31 (1.01-1.67)	1.57 (1.21-2.00)	2.00 (1.53-2.56)	2.35 (1.80-3.02)	2.84 (2.11-3.78)	3.20 (2.34-4.34)	3.59 (2.55-5.01)	4.04 (2.72-5.70)	4.70 (3.05-6.81)	5.26 (3.34-7.73)
3-hr	1.52 (1.18-1.93)	1.83 (1.41-2.32)	2.32 (1.79-2.95)	2.73 (2.09-3.49)	3.29 (2.46-4.37)	3.71 (2.72-5.01)	4.16 (2.98-5.80)	4.69 (3.17-6.59)	5.48 (3.56-7.90)	6.14 (3.91-9.00)
6-hr	1.95 (1.52-2.45)	2.33 (1.81-2.94)	2.96 (2.29-3.74)	3.48 (2.68-4.41)	4.19 (3.15-5.52)	4.73 (3.48-6.34)	5.30 (3.81-7.33)	5.98 (4.05-8.32)	6.99 (4.56-9.99)	7.84 (5.00-11.4)
12-hr	2.42 (1.90-3.03)	2.90 (2.28-3.64)	3.69 (2.88-4.63)	4.34 (3.37-5.47)	5.24 (3.95-6.84)	5.90 (4.37-7.85)	6.62 (4.77-9.07)	7.45 (5.07-10.3)	8.70 (5.70-12.3)	9.75 (6.24-14.0)
24-hr	2.85 (2.25-3.54)	3.44 (2.71-4.28)	4.40 (3.46-5.49)	5.20 (4.07-6.51)	6.31 (4.79-8.19)	7.13 (5.31-9.42)	8.01 (5.82-10.9)	9.06 (6.18-12.4)	10.6 (7.00-15.0)	12.0 (7.71-17.1)
2-day	3.18 (2.53-3.92)	3.88 (3.09-4.80)	5.04 (3.99-6.24)	6.00 (4.72-7.46)	7.32 (5.60-9.47)	8.30 (6.24-10.9)	9.36 (6.87-12.8)	10.7 (7.32-14.5)	12.7 (8.39-17.8)	14.5 (9.34-20.5)
3-day	3.44 (2.75-4.23)	4.21 (3.36-5.18)	5.46 (4.34-6.74)	6.50 (5.14-8.05)	7.94 (6.10-10.2)	8.99 (6.78-11.8)	10.1 (7.47-13.8)	11.6 (7.95-15.7)	13.8 (9.12-19.2)	15.8 (10.2-22.2)
4-day	3.70 (2.96-4.53)	4.50 (3.60-5.52)	5.82 (4.64-7.15)	6.91 (5.47-8.53)	8.41 (6.47-10.8)	9.52 (7.19-12.4)	10.7 (7.91-14.5)	12.2 (8.41-16.5)	14.5 (9.62-20.1)	16.6 (10.7-23.2)
7-day	4.41 (3.55-5.38)	5.29 (4.25-6.45)	6.71 (5.38-8.21)	7.90 (6.29-9.69)	9.53 (7.36-12.1)	10.7 (8.13-13.9)	12.0 (8.89-16.1)	13.6 (9.41-18.2)	16.0 (10.6-22.0)	18.1 (11.7-25.2)
10-day	5.12 (4.14-6.21)	6.03 (4.87-7.33)	7.52 (6.05-9.16)	8.76 (7.01-10.7)	10.5 (8.11-13.2)	11.7 (8.90-15.1)	13.1 (9.66-17.4)	14.7 (10.2-19.6)	17.1 (11.4-23.4)	19.1 (12.4-26.5)
20-day	7.27 (5.92-8.76)	8.26 (6.71-9.96)	9.87 (8.00-11.9)	11.2 (9.03-13.6)	13.1 (10.2-16.3)	14.5 (11.0-18.3)	15.9 (11.7-20.7)	17.5 (12.2-23.1)	19.7 (13.2-26.7)	21.5 (14.0-29.5)
30-day	9.08 (7.42-10.9)	10.1 (8.26-12.1)	11.8 (9.60-14.2)	13.2 (10.7-16.0)	15.1 (11.8-18.7)	16.6 (12.6-20.9)	18.1 (13.3-23.2)	19.6 (13.7-25.8)	21.7 (14.6-29.2)	23.3 (15.2-31.8)
45-day	11.3 (9.31-13.5)	12.4 (10.2-14.9)	14.2 (11.6-17.0)	15.7 (12.7-18.8)	17.7 (13.8-21.8)	19.3 (14.7-24.0)	20.8 (15.2-26.4)	22.3 (15.6-29.1)	24.1 (16.2-32.3)	25.4 (16.6-34.6)
60-day	13.2 (10.9-15.8)	14.4 (11.8-17.1)	16.2 (13.3-19.3)	17.7 (14.4-21.2)	19.8 (15.5-24.3)	21.5 (16.4-26.6)	23.0 (16.9-29.0)	24.5 (17.2-31.8)	26.2 (17.7-34.9)	27.3 (17.9-37.0)

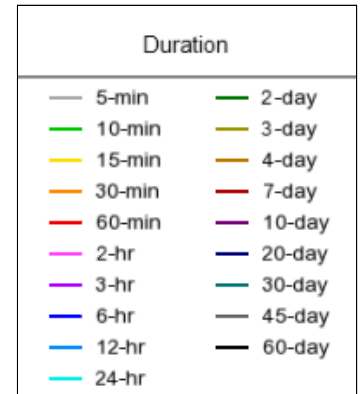
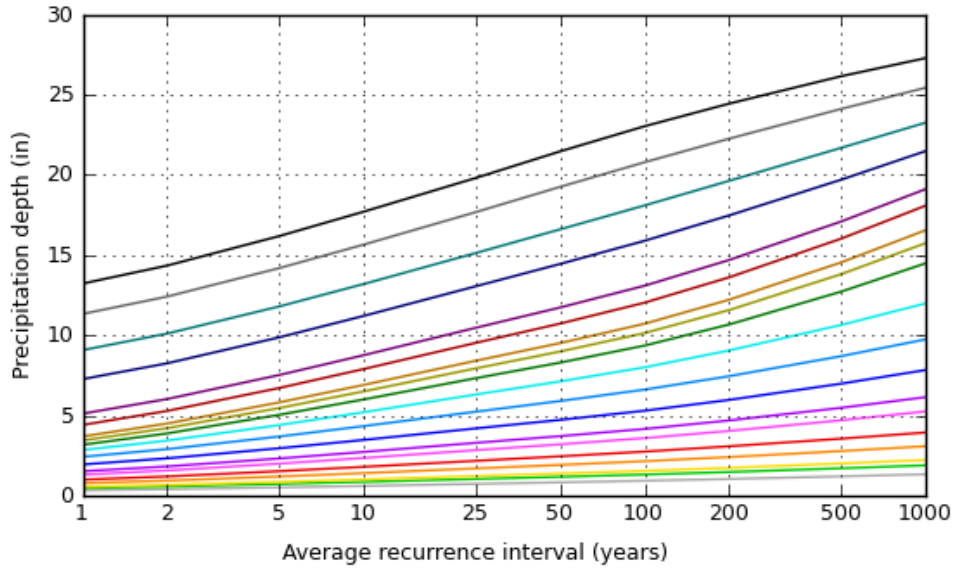
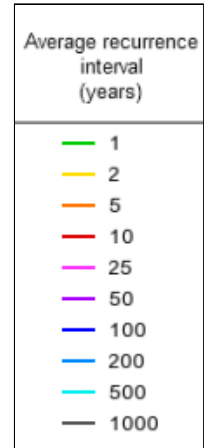
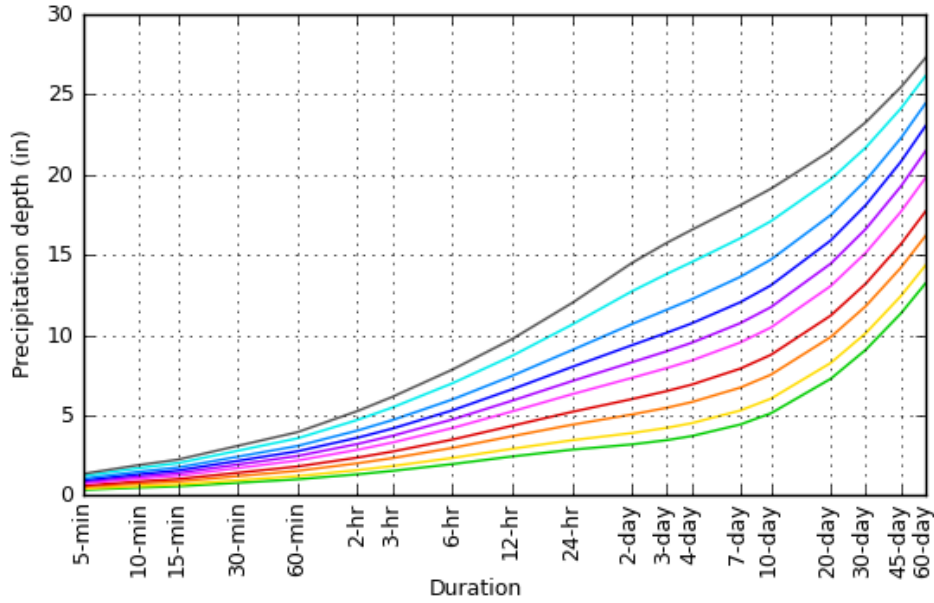
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves

Latitude: 41.3468°, Longitude: -72.4094°



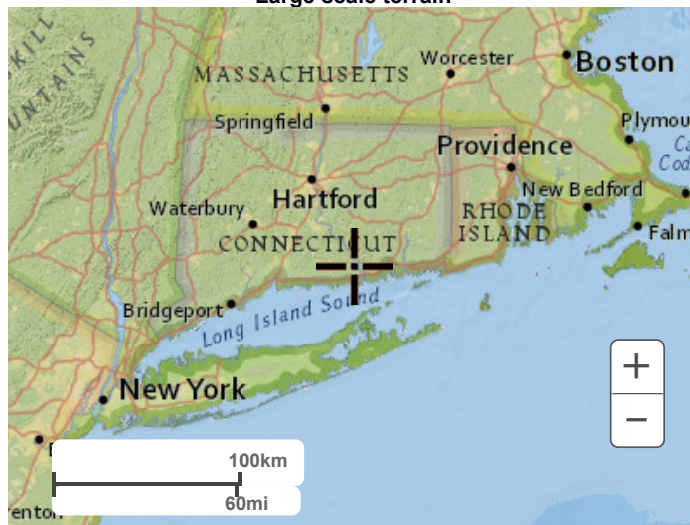
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Maps & aerials

Small scale terrain



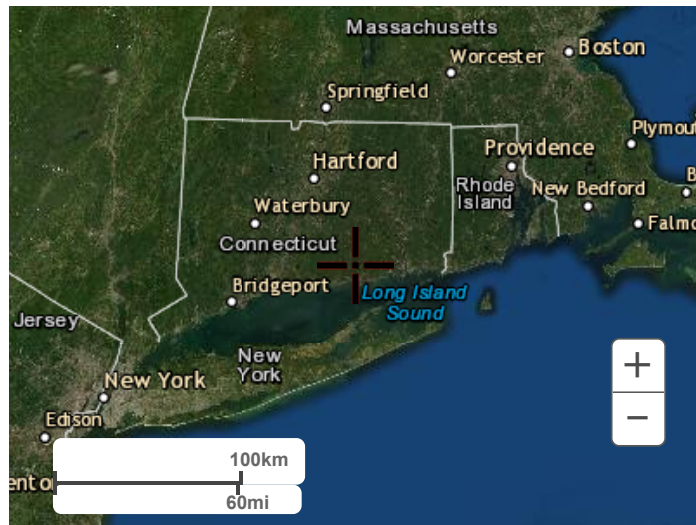
Large scale terrain



Large scale map



Large scale aerial



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[National Water Center](#)
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

[Disclaimer](#)



NOAA Atlas 14, Volume 10, Version 3
Location name: Essex, Connecticut, USA*
Latitude: 41.3468°, Longitude: -72.4094°
Elevation: 35.92 ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	4.06 (3.11-5.20)	4.87 (3.72-6.24)	6.20 (4.73-7.98)	7.31 (5.54-9.44)	8.83 (6.50-11.8)	9.97 (7.22-13.6)	11.2 (7.87-15.7)	12.5 (8.40-17.8)	14.5 (9.35-21.1)	16.0 (10.2-23.8)
10-min	2.87 (2.20-3.68)	3.45 (2.64-4.42)	4.40 (3.35-5.66)	5.18 (3.93-6.69)	6.25 (4.61-8.36)	7.06 (5.11-9.61)	7.91 (5.58-11.1)	8.87 (5.95-12.6)	10.2 (6.63-15.0)	11.4 (7.19-16.9)
15-min	2.25 (1.72-2.89)	2.70 (2.07-3.47)	3.44 (2.63-4.43)	4.06 (3.08-5.24)	4.90 (3.62-6.56)	5.54 (4.01-7.54)	6.21 (4.38-8.70)	6.96 (4.66-9.89)	8.04 (5.20-11.7)	8.92 (5.64-13.2)
30-min	1.56 (1.20-2.00)	1.88 (1.44-2.41)	2.39 (1.82-3.08)	2.81 (2.14-3.64)	3.40 (2.51-4.55)	3.84 (2.78-5.22)	4.30 (3.03-6.03)	4.82 (3.23-6.85)	5.56 (3.60-8.13)	6.17 (3.91-9.16)
60-min	1.00 (0.766-1.28)	1.20 (0.919-1.54)	1.53 (1.17-1.97)	1.80 (1.37-2.32)	2.17 (1.60-2.91)	2.46 (1.78-3.34)	2.75 (1.94-3.85)	3.08 (2.07-4.38)	3.56 (2.30-5.19)	3.94 (2.50-5.85)
2-hr	0.656 (0.506-0.835)	0.786 (0.606-1.00)	0.999 (0.767-1.28)	1.18 (0.898-1.51)	1.42 (1.05-1.89)	1.60 (1.17-2.17)	1.80 (1.28-2.51)	2.02 (1.36-2.85)	2.35 (1.53-3.41)	2.63 (1.67-3.87)
3-hr	0.507 (0.393-0.644)	0.608 (0.470-0.772)	0.773 (0.595-0.983)	0.909 (0.697-1.16)	1.10 (0.818-1.46)	1.24 (0.905-1.67)	1.39 (0.991-1.93)	1.56 (1.05-2.19)	1.83 (1.19-2.63)	2.05 (1.30-3.00)
6-hr	0.325 (0.253-0.410)	0.389 (0.303-0.491)	0.494 (0.383-0.624)	0.581 (0.448-0.737)	0.700 (0.525-0.922)	0.789 (0.581-1.06)	0.885 (0.636-1.22)	0.998 (0.676-1.39)	1.17 (0.761-1.67)	1.31 (0.835-1.90)
12-hr	0.201 (0.158-0.252)	0.241 (0.189-0.302)	0.306 (0.239-0.384)	0.360 (0.280-0.454)	0.435 (0.328-0.568)	0.490 (0.363-0.651)	0.549 (0.396-0.753)	0.619 (0.421-0.855)	0.722 (0.473-1.02)	0.809 (0.518-1.17)
24-hr	0.119 (0.094-0.147)	0.143 (0.113-0.178)	0.183 (0.144-0.229)	0.217 (0.170-0.271)	0.263 (0.200-0.341)	0.297 (0.221-0.392)	0.334 (0.242-0.455)	0.377 (0.258-0.518)	0.444 (0.291-0.624)	0.500 (0.321-0.713)
2-day	0.066 (0.053-0.082)	0.081 (0.064-0.100)	0.105 (0.083-0.130)	0.125 (0.098-0.155)	0.153 (0.117-0.197)	0.173 (0.130-0.228)	0.195 (0.143-0.266)	0.222 (0.152-0.303)	0.265 (0.175-0.370)	0.302 (0.195-0.427)
3-day	0.048 (0.038-0.059)	0.058 (0.047-0.072)	0.076 (0.060-0.094)	0.090 (0.071-0.112)	0.110 (0.085-0.142)	0.125 (0.094-0.164)	0.141 (0.104-0.191)	0.161 (0.110-0.218)	0.192 (0.127-0.266)	0.219 (0.141-0.308)
4-day	0.039 (0.031-0.047)	0.047 (0.038-0.058)	0.061 (0.048-0.074)	0.072 (0.057-0.089)	0.088 (0.067-0.112)	0.099 (0.075-0.130)	0.112 (0.082-0.151)	0.127 (0.088-0.172)	0.152 (0.100-0.210)	0.172 (0.112-0.242)
7-day	0.026 (0.021-0.032)	0.031 (0.025-0.038)	0.040 (0.032-0.049)	0.047 (0.037-0.058)	0.057 (0.044-0.072)	0.064 (0.048-0.083)	0.072 (0.053-0.096)	0.081 (0.056-0.109)	0.095 (0.063-0.131)	0.108 (0.070-0.150)
10-day	0.021 (0.017-0.026)	0.025 (0.020-0.031)	0.031 (0.025-0.038)	0.037 (0.029-0.045)	0.044 (0.034-0.055)	0.049 (0.037-0.063)	0.055 (0.040-0.072)	0.061 (0.042-0.082)	0.071 (0.047-0.097)	0.080 (0.052-0.110)
20-day	0.015 (0.012-0.018)	0.017 (0.014-0.021)	0.021 (0.017-0.025)	0.023 (0.019-0.028)	0.027 (0.021-0.034)	0.030 (0.023-0.038)	0.033 (0.024-0.043)	0.036 (0.025-0.048)	0.041 (0.027-0.056)	0.045 (0.029-0.062)
30-day	0.013 (0.010-0.015)	0.014 (0.011-0.017)	0.016 (0.013-0.020)	0.018 (0.015-0.022)	0.021 (0.016-0.026)	0.023 (0.018-0.029)	0.025 (0.018-0.032)	0.027 (0.019-0.036)	0.030 (0.020-0.041)	0.032 (0.021-0.044)
45-day	0.010 (0.009-0.013)	0.011 (0.009-0.014)	0.013 (0.011-0.016)	0.014 (0.012-0.017)	0.016 (0.013-0.020)	0.018 (0.014-0.022)	0.019 (0.014-0.024)	0.021 (0.014-0.027)	0.022 (0.015-0.030)	0.024 (0.015-0.032)
60-day	0.009 (0.008-0.011)	0.010 (0.008-0.012)	0.011 (0.009-0.013)	0.012 (0.010-0.015)	0.014 (0.011-0.017)	0.015 (0.011-0.018)	0.016 (0.012-0.020)	0.017 (0.012-0.022)	0.018 (0.012-0.024)	0.019 (0.012-0.026)

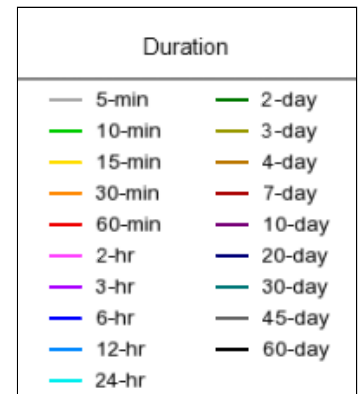
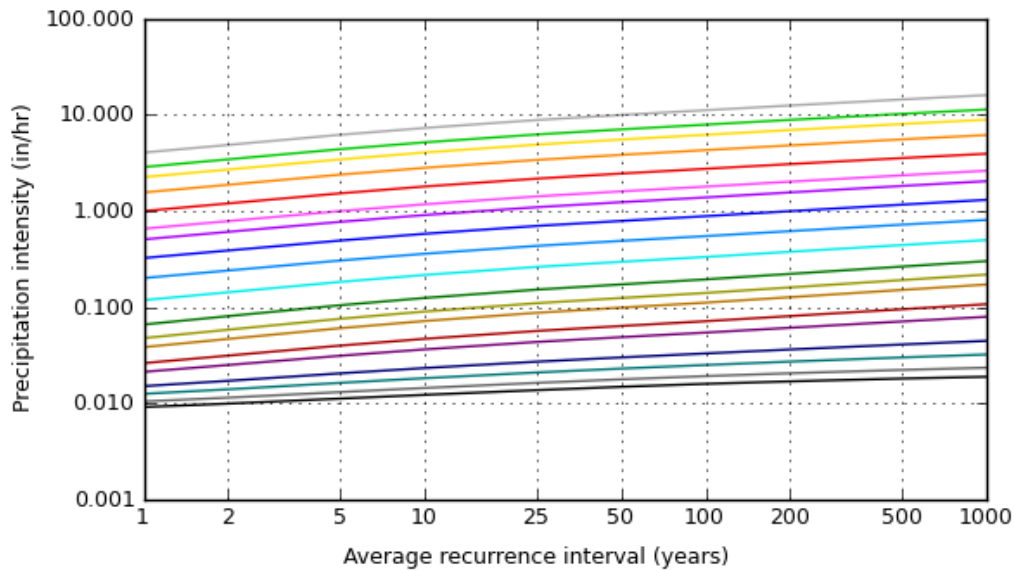
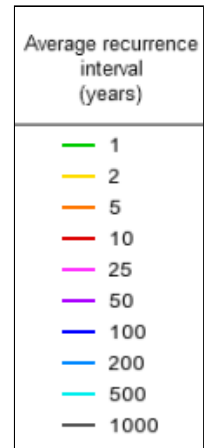
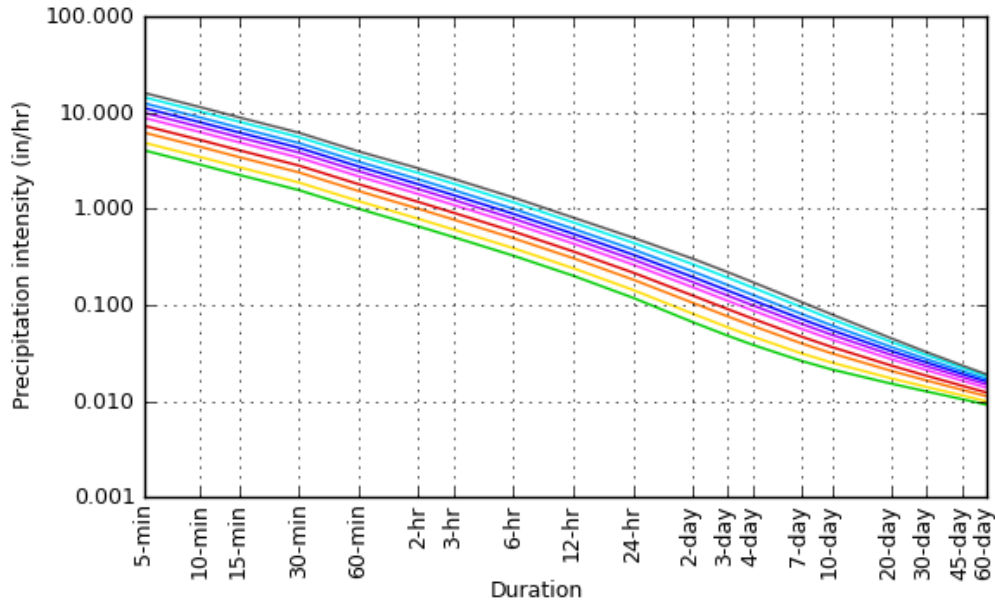
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based intensity-duration-frequency (IDF) curves

Latitude: 41.3468°, Longitude: -72.4094°



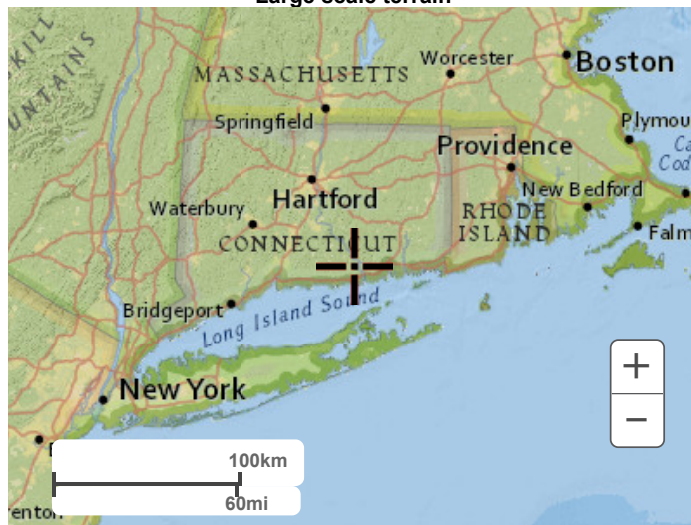
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Maps & aerials

Small scale terrain



Large scale terrain



Large scale map



Large scale aerial



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Appendix F
Domestic Water Usage Data

Water Data Usage 49 Plains Road

Date	Meter Reading Gallons Used	Number of Days	Gallons Per Day
6/6/2019	7000	97	72
9/6/2019	13000	92	141
10/23/2019	5000	47	106
12/6/2019	3000	44	68
3/4/2020	7000	89	79
6/2/2020	8000	90	89
9/4/2020	11000	94	117
12/8/2020	10000	95	105
3/4/2021	13000	86	151
6/4/2021	44000	92	478
9/9/2021	40000	97	412
12/9/2021	7000	91	77
3/7/2022	157000	88	1784
3/9/2022	4000	2	2000
6/7/2022	29000	90	322
9/7/2022	8000	92	87
Average Gallons Per Day			144

Appendix G
Ground Water Monitoring Data

GROUNDWATER MONITORING

Prepared for
49 PLAINS ROAD
ESSEX, CT.

TEST HOLE	#4		
ELEVATION TOP OF PIPE =		40.77'(-3.65')	
ELEVATION OF GROUND =		37.12'	
DATE	TOP OF PIPE TO GROUNDWATER	SURFACE TO GROUNDWATER	WATER ELEV.
1/21/22	7.80'	4.15'	32.97'
2/2/22	8.20'	4.55'	32.57'
2/11/22	7.80'	4.15'	32.97'
2/22/22	6.80'	3.15'	33.97'
3/4/22	6.70'	3.05'	34.07'
3/15/22	6.70'	3.05'	34.07'
3/29/22	7.00'	3.35'	33.77'
4/18/22	7.20'	3.55'	33.57
5/17/22	7.30'	3.65'	33.47'

TEST HOLE	#3		
ELEVATION TOP OF PIPE =		40.52'(3.57')	
ELEVATION OF GROUND =		36.95	
DATE	TOP OF PIPE TO GROUNDWATER	SURFACE TO GROUNDWATER	WATER ELEV.
1/21/22	7.60'	4.03'	32.92'
2/2/22	8.00'	4.43'	32.52'
2/11/22	7.70'	4.13'	32.82
2/22/22	6.80'	3.23'	33.72'
3/4/22	6.90'	3.33'	33.62'
3/15/22	6.60'	3.03'	33.92'
3/29/22	6.90'	3.33'	33.62'
4/18/22	7.10'	3.53'	33.42'
5/17/22	7.30'	3.73'	33.22'

GROUNDWATER MONITORING

Prepared for
49 PLAINS ROAD
ESSEX, CT.

TEST HOLE	#1		
ELEVATION TOP OF PIPE =		40.48'(3.43')	
ELEVATION OF GROUND =		37.05	
DATE	TOP OF PIPE TO GROUNDWATER	SURFACE TO GROUNDWATER	WATER ELEV.
1/21/22	7.80'	4.37'	32.68'
2/2/22	8.30'	4.87'	32.18'
2/11/22	7.10'	3.67'	33.38'
2/22/22	7.00'	3.57'	33.48'
3/4/22	6.90'	3.47'	33.58'
3/15/22	6.90'	3.47'	33.58'
3/29/22	7.10'	3.67'	33.38'
4/18/22	7.30'	3.87'	33.18'
5/17/22	7.40'	3.97'	33.08'

TEST HOLE	#10		
ELEVATION TOP OF PIPE =		39.26(-3.40')	
ELEVATION OF GROUND =		35.86	
DATE	TOP OF PIPE TO GROUNDWATER	SURFACE TO GROUNDWATER	WATER ELEV.
1/21/22	7.60'	4.20'	31.66'
2/2/22	8.00'	4.60'	31.26'
2/11/22	7.00'	3.60'	32.26'
2/22/22	6.60'	3.20'	32.66'
3/4/22	6.70'	3.30'	32.56'
3/15/22	6.60'	3.20'	32.66'
3/29/22	6.90'	3.50'	32.36'
4/18/22	7.00'	3.60'	32.26'
5/17/22	7.00'	3.60'	32.26'

GROUNDWATER MONITORING

Prepared for
49 PLAINS ROAD
ESSEX, CT.

TEST HOLE	#9		
ELEVATION TOP OF PIPE =		38.09'(-1.40')	
ELEVATION OF GROUND =		37.09	
DATE	TOP OF PIPE TO GROUNDWATER	SURFACE TO GROUNDWATER	WATER ELEV.
1/21/22	6.50'	5.10'	31.99'
2/2/22	6.80'	5.40'	31.69'
2/11/22	6.00'	4.60'	32.49'
2/22/22	6.00'	4.60'	32.49'
3/4/22	6.10'	4.70'	32.39'
3/15/22	5.90'	4.50'	32.59'
3/29/22	6.20'	4.80'	32.29'
4/18/22	6.30'	4.90'	32.19'
5/17/22	6.20'	4.80'	32.29'

Appendix H
Soil Infiltration Rates

Permeability Rates

Sample A

No infiltration rate was determined soil was not suitable for Infiltration

Sample B

No infiltration rate was determined soil was not suitable for Infiltration

Sample C

Sample	Sample Round	L (inches)	H1 (inches)	H2 (inches)	t (min)	t (hours)	K (in/hr)	K (ft/day)
C TP 13 Depth 38" Trial 1	1	4	16	14.7	1.00	0.017	20.33	40.651
	2	4	14.7	13.5	1.00	0.017	20.43	40.851
	3	4	13.5	12.4	1.00	0.017	20.39	40.772
	4	4	12.4	11.5	1.00	0.017	18.08	36.151
	5	4	11.5	10.6	1.00	0.017	19.55	39.095
	6	4	10.6	9.8	1.00	0.017	18.82	37.647
	7	4	9.8	9.1	1.00	0.017	17.78	35.556
	8	4	9.1	8.5	1.00	0.017	16.36	32.727
	9	4	8.5	7.8	1.00	0.017	20.61	41.227
	10	4	7.8	7.3	1.00	0.017	15.89	31.788
	10	4	7.3	6.7	1.00	0.017	20.57	41.143
	10	4	6.7	6.3	1.00	0.017	14.77	29.538
	sample average							18.631

Sample	Sample Round	L (inches)	H1 (inches)	H2 (inches)	t (min)	t (hours)	K (in/hr)	K (ft/day)
C TP 13 Depth 38" Trial 2	1	4	16	13	2.00	0.033	24.83	49.655
	2	4	13	11	2.00	0.033	20.00	40.000
	3	4	11	9.2	2.00	0.033	21.39	42.772
	4	4	9.2	7.7	2.00	0.033	21.30	42.604
	5	4	7.7	6.4	2.00	0.033	22.13	44.255
	sample average							21.929

Sample D

Sample	Sample Round	L (inches)	H1 (inches)	H2 (inches)	t (min)	t (hours)	K (in/hr)	K (ft/day)
D TP 13 Depth 38" Trial 1	1	5	16	15.1	1.00	0.017	17.36	34.727
	2	5	15.1	14.1	1.00	0.017	20.55	41.096
	3	5	14.1	13.2	1.00	0.017	19.78	39.560
	4	5	13.2	12.4	1.00	0.017	18.75	37.500
	5	5	12.4	11.6	1.00	0.017	20.00	40.000
	6	5	11.6	10.9	1.00	0.017	18.67	37.333
	7	5	10.9	10.2	1.00	0.017	19.91	39.810
	8	5	10.2	9	2.00	0.033	18.75	37.500
	9	5	9	7.9	2.00	0.033	19.53	39.053
	10	5	7.9	7	2.00	0.033	18.12	36.242
	sample average							19.141

Sample	Sample Round	L (inches)	H1 (inches)	H2 (inches)	t (min)	t (hours)	K (in/hr)	K (ft/day)
D TP 13 Depth 38" Trial 2	1	5	16	13.3	2.00	0.033	27.65	55.290
	2	5	13.3	12.9	2.00	0.033	4.58	9.160
	3	5	12.9	11.6	2.00	0.033	15.92	31.837
	4	5	11.6	10.3	2.00	0.033	17.81	35.616
	5	5	10.3	9.3	2.00	0.033	15.31	30.612
	6	5	9.3	8.4	2.00	0.033	15.25	30.508
	7	5	8.4	7.4	2.00	0.033	18.99	37.975
	8	5	7.4	6.6	2.00	0.033	17.14	34.286
	sample average							16.580

Sample E

Sample	Sample Round	L (inches)	H1 (inches)	H2 (inches)	t (min)	t (hours)	K (in/hr)	K (ft/day)
E TP 14 Depth 30"	1	5	16	15.6	30.00	0.500	0.25	0.506
	2	5	15.6	15.2	30.00	0.500	0.26	0.519
	3	5	15.2	14.7	30.00	0.500	0.33	0.669
	4	5	14.7	14.2	30.00	0.500	0.35	0.692
	5	5	14.2	13.8	30.00	0.500	0.29	0.571
	6	5	13.8	13.5	30.00	0.500	0.22	0.440
	7	5	13.5	13	30.00	0.500	0.38	0.755
	8	5	13	12.3	60.00	1.000	0.28	0.553
	9	5	12.3	10.8	120.00	2.000	0.32	0.649
	sample average							0.298

Sample F

Sample	Sample Round	L (inches)	H1 (inches)	H2 (inches)	t (min)	t (hours)	K (in/hr)	K (ft/day)
F TP 14 Depth 30"	1	5.25	16	15.1	30.00	0.500	0.61	1.215
	2	5.25	15.1	14.2	30.00	0.500	0.65	1.290
	3	5.25	14.2	13.4	30.00	0.500	0.61	1.217
	4	5.25	13.4	12.5	30.00	0.500	0.73	1.459
	5	5.25	12.5	11.8	30.00	0.500	0.60	1.210
	6	5.25	11.8	11.2	30.00	0.500	0.55	1.096
	7	5.25	11.2	10.5	30.00	0.500	0.68	1.355
	8	5.25	10.5	9.4	60.00	1.000	0.58	1.161
	9	5.25	9.4	7.4	120.00	2.000	0.63	1.250
	sample average							0.625